# COASTAL <br> <br> ENGINEERED WOOD SOLUTIONS 

 <br> <br> ENGINEERED WOOD SOLUTIONS}

Quality Products \& Design Assistance You Can Build On


## TECHNICAL DATA GUIDE

NORDIC® ${ }^{\oplus}$-JOIST I NORBORD ${ }^{\circledR}$ RIM BOARD \| TRIFORCE ${ }^{\oplus}$ OPEN JOISTS \| CP-LAM LVL \| PWT TREATED ${ }^{\text {w }}$ LVL ANTHONY® BEAMS \& COLUMNS \| PWT FRAMING LUMBER \| SIMPSON® ENGINEERED CONNECTORS

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## SOLID-SAWN JOIST <br> DESIGN PROPERTIES

Chantiers Chibougamau Ltd. harvests its own trees, which enables Nordic products to adhere to strict quality control procedures throughout the manufacturing process. Every phase of the operation, from forest to the finished product, reflects our commitment to quality
Nordic ${ }^{\oplus}$ Engineered Wood I-joists use only finger-jointed black spruce lumber in their flanges ensuring consistent quality, superior strength, and longer span carrying capacity.


DESIGN PROPERTIES FOR NORDIC® ${ }^{\circledR}$-JOISTS
(a) (b)

| JOIST DEPTH | JOIST <br> SERIES | $\left(10^{6} \mathrm{El}(\mathrm{c})\right.$ | $\begin{gathered} M^{(d)} \\ (\mathrm{lbf}-\mathrm{ft}) \end{gathered}$ | $\begin{aligned} & V^{(\mathrm{e})} \\ & (\mathrm{lbf}) \end{aligned}$ | $\begin{aligned} & \mathrm{IR}^{(\mathrm{f})} \\ & (\mathrm{I} \mathrm{bf}) \end{aligned}$ | IR w/WS(g) ( bf ) | $\begin{gathered} E R^{(h)} \\ (\mathrm{lbf}) \end{gathered}$ | $\begin{gathered} \mathrm{K}^{(i)} \\ \left(10^{6} \mathrm{lbf}\right) \end{gathered}$ | WEIGHT ( $\mathrm{lbf} / \mathrm{ft}$ ) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | NI-40x | 218 | 2,900 | 1,200 | 2,240 | 2,620 | 1,120 | 4.94 | 2.65 |
| 9-1/2 | NI-80 | 324 | 5,385 | 1,200 | 2,380 | 2,790 | 1,190 | 4.94 | 3.27 |
| 11-7/8" | NI-40x | 371 | 3,760 | 1,480 | 2,750 | 2,930 | 1,250 | 6.18 | 2.85 |
|  | NI-60 | 396 | 4,935 | 1,480 | 2,750 | 2,930 | 1,250 | 6.18 | 2.99 |
|  | NI-80 | 547 | 6,980 | 1,480 | 2,900 | 3,120 | 1,330 | 6.18 | 3.45 |
|  | NI-90 | 601 | 8,780 | 1,925 | 3,670 | 3,670 | 1,400 | 6.18 | 3.45 |
| 14" | NI-60 | 584 | 5,945 | 1,730 | 2,750 | 3,240 | 1,250 | 7.28 | 3.15 |
|  | $\mathrm{NI}-80$ | 802 | 8,405 | 1,730 | 3,310 | 3,840 | 1,330 | 7.28 | 3.75 |
|  | NI-90 | 877 | 10,570 | 2,125 | 3,820 | 3,820 | 1,690 | 7.28 | 3.75 |
| 16" | NI-60 | 799 | 6,895 | 1,970 | 2,750 | 3,240 | 1,250 | 8.32 | 3.46 |
|  | NI-80 | 1,092 | 9,745 | 1,970 | 3,310 | 3,840 | 1,330 | 8.32 | 3.95 |
|  | NI-90 | 1,187 | 12,260 | 2,330 | 3,930 | 3,930 | 1,875 | 8.32 | 3.95 |

Highlighted sizes indicates stocked depths.

For SI: $1 \mathrm{lbf}=4.448 \mathrm{~N}, \quad 1 \mathrm{lbf}-\mathrm{in} 2=0.00287 \mathrm{~N}-\mathrm{m} 2, \quad 1 \mathrm{inch}=25.4 \mathrm{~mm}$.
(a) The tabulated values are design values for normal duration of load. All values, except for El and K, are permitted to be adjusted for other load durations as permitted by the code for solid sawn lumber.
(b) The vertical (bearing) load capacity is $2,000 \mathrm{lb} / \mathrm{ft}$ without bearing stiffeners.
(c) Bending stiffness (EI) of the I-joist.
(d) Moment capacity (M) of the I-joist, which shall not be increased by any code allowed repetitive member use factor.
(e) Shear capacity (V) of the I-joist.
(f) Intermediate reaction (IR) of the I-joist with a minimum bearing length of 3-1/2 inches without bearing stiffeners.
(g) Intermediate reaction (IR w/WS) of the I-joist with a minimum bearing length of 3-1/2 inches with bearing stiffeners.
(h) End reaction (ER) of the I-joist with a minimum bearing length of 1-3/4 inches without bearing stiffeners. Higher end reactions are permitted. For a bearing length of 4 inches, the end reaction may be set equal to the tabulated shear value. Interpolation of the end reaction between $1-3 / 4$ and 4 -inch bearing is permitted. For end reaction values over 1,550 lbf, bearing stiffeners are required.
(i) Coefficient of shear deflection (K). For calculating uniform load and center-point load deflections of the I-joist in a simple-span application, use Eqs. 1 and 2.

> Uniform Load:

Center-Point Load:
$\delta$
$=\frac{P \ell^{3}}{48 E I}$ $+$
$\frac{2 P l}{K}$

Where:

$$
\begin{aligned}
& \delta=\text { calculated deflection (in.) } \\
& \omega=\text { uniform load (lbf/in.) } \\
& \ell=\text { design span (in.) } \\
& P=\text { concentrated load (lbf) } \\
& E I=\text { bending stiffness of the I-joist (lbf-in.2) } \\
& K=\text { coefficient of shear deflection (lbf) }
\end{aligned}
$$

## SOLID-SAWN JOIST

ALLOWABLE FLOORSPANS

ALLOWABLE FLOOR SPANS - Live Load $=40$ psf, Dead Load $=10$ psf
Live Load Deflection Limit of L/480

| JOIST <br> DEPTH | JOISTSERIES | SIMPLE SPANS |  |  |  | MULTTPLE SPANS |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | ON CENTER SPACING |  |  |  | ON CENTER SPACING |  |  |  |
|  |  | 12" | 16" | 19.2" | 24" | 12" | $16^{\prime \prime}$ | 19.2" | 24" |
| 9-1/2" | NI-40x | 18'-8" | 17'-0" | 16-1" | 15'-0" | 20'-4" | 18'-5" | 16'-10" | 15'-0" |
|  | $\mathrm{NI}-80$ | 20'-11" | 19'-1" | 18'-0" | 16'-9" | 22'-9" | 20'-9" | 19'-6" | 18'-2" |
| 11-7/8" | NI-40x | 22'-2" | 20'-3" | 19'-2" | 17'-2" | 24'-2" | 21'-0" | 19'-2" | 17'-1" |
|  | NI-60 | 22'-8" | 20'-8" | 19'-6" | 18'-2" | 24'-8" | 22'-6" | 21'-2" | 19'-8" |
|  | NI-80 | 24'-11" | 22'-8" | 21'-4" | 19'-11" | 27'-1" | 24'-8" | 23'-3" | 21'-7" |
|  | NI-90 | 25'-7" | 23'-3" | 21'-11" | 20'-5" | 27'-10" | 25'-4" | 23'-10" | 22'-2" |
| 14" | NI-60 | 25'-9" | 23'-6" | 22'-2" | 20'-8" | 28'-0" | 25'-7" | 24'-1" | 21'-7" |
|  | $\mathrm{NI}-80$ | 28'-3" | 25'-9" | 24'-3" | 22'-7" | 30'-10" | 28'-0" | 26'-5" | 24'-6" |
|  | NI-90 | 29'-0" | 26'-5" | 24'-10" | 23'-1" | 31'-7" | 28'-9" | 27'-1" | 25'-2" |
| 16" | NI-60 | 28'-6" | 26'-0" | 24'-7" | 22'-10" | 31'-1" | 28'-4" | 26'-0" | 21'-9" |
|  | $\mathrm{Nl}-80$ | 31'-4" | 28'-6" | 26'-10" | 25'-0" | 34'-2" | 31'-1" | 29'-3" | 26'-3" |
|  | NI-90 | 32'-1" | 29'-3" | 27'-6" | 25'-7" | 35'-0" | 31'-10" | 29'-11' | 27'-10" |

## Highlighted sizes indicates stocked depths.

## NOTES:

1. Allowable clear span applicable to residential floor construction with a design live load of 40 psf and dead load of 10 psf . The live load deflection is limited to $L / 480$ as shown, and the total load deflection to $L / 360$. For multiple-span applications, the end spans shall be $40 \%$ or more of the adjacent span.
2. Spans are based on a composite floor with glued-nailed sheathing meeting the requirements for APA Rated Sheathing or APA Rated STURD-I-FLOOR conforming to PRP-108, PS 1, or PS 2 with a minimum thickness of $19 / 32$ inch ( $40 / 20$ or 20 o.c.) for a joist spacing of 19.2 inches or less, or $23 / 32$ inch ( $48 / 24$ or 24 o.c.) for a joist spacing of 24 inches. Adhesive shall meet APA Specification AFG-01 or ASTM D3498.
3. Minimum bearing length shall be $1-3 / 4$ inches for the end bearings, and $3-1 / 2$ inches for the intermediate bearings.
4. Bearing stiffeners are not required when I-joists are used with the spans and spacing given in these tables, except as required for hangers.
5. These span charts are based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties.
6. For ceramic tile applications, spacings greater than 16 " o.c. are typically not recommended.


## CPI-90 JOIST <br> DIMENSIONS \& SPANS



7/16" OSB Web
$3-1 / 2^{\prime \prime} \times 1-1 / 2^{\prime \prime}$ Flange


| CPI-90 | I-Joist Depth | Coastal Code | $\begin{aligned} & \hline \text { APA } \\ & \text { Code } \end{aligned}$ | $\begin{gathered} \mathrm{El}^{(4)} \\ \left(\mathbf{X 1 0} 1 \mathrm{lb}-\mathrm{in}^{2}\right) \end{gathered}$ | $\begin{gathered} M^{(5)} \\ (\mathrm{ft}-\mathrm{lb}) \end{gathered}$ | $V^{(6)}$ <br> (lb) | $\begin{aligned} & \hline \mathbf{I R}^{(7)} \\ & (\mathrm{Ib}) \\ & \hline \end{aligned}$ | $\begin{gathered} \hline \mathrm{ER}^{(8)} \\ \text { (lb) } \end{gathered}$ | $\begin{gathered} \mathbf{K}^{(9)} \\ \left(\mathbf{X 1 0} 0^{6} \mathrm{lb}\right) \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 11-7/8" | CPI 9012 | PRI-90 | 661 | 10255 | 1925 | 3355 | 1400 | 6.92 |
|  | 16 | CPI 9016 | PRI-90 | 1306 | 14020 | 2330 | 3355 | 1400 | 9.35 |

Highlighted sizes indicates stocked depths.

1. The tabulated design properties are for normal duration of load. All properties, except El and k , may be adjusted for other load durations as permitted by the code.
2. PRI-90 joist series designation. Design properties meet or exceed the requirements of the PRI-90 Performance Standard for APA EWS I-joist.
3. Coastal Forest Products Corporation proprietary joist series designation.
4. Bending stiffness (EI)
5. Moment capacity (M). The tabulated values shall not be increased by any code-allowed repetitive member factor.
6. Shear capacity (V).
7. Intermediate reaction capacity (iR) of the immediate I-joist without web stiffeners and a minimum bearing length of 3-1/2 inches.
8. End reaction capacity (ER) of the I-joist without web stiffeners and a minimum bearing length of $1-3 / 4$ inches.
9. Coefficient of shear deflection (k). Use equations 1 or 2 to calculate uniform load or center point load deflections in a simple-span application.

Uniform Load
Center-Point Load:
$(1) \delta=5 \frac{\omega \ell^{4}+}{384 E I} \quad \frac{\omega \ell^{2}}{K}$
(2) $\delta=\frac{P \ell^{3}}{48 E I}+\frac{2 P \ell}{K}$

Where: $\delta=$ calculated deflection (in.)
$\omega=$ uniform load (lbf/in.)
$\ell=$ design span (in.)
$P=$ concentrated load (lbf)
$E I=$ bending stiffness of the CPI-joist (lbf-in.2)
10. $2 \times 4$ web stiffeners required. Attach with 10 nails ( $3-1 / 2$ " long $\times 0.131$ " diameter)
$K=$ coefficient of shear deflection (lbf)
11. $2 \times 4$ web stiffeners required. Attach with 8 nails ( $3-1 / 2$ " long $\times 0.131$ " diameter)

|  | Allowable Floor Spans |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Simple Spans |  |  |  |  | Multiple Spans |  |  |  |
| CPI Joist Series | CPI Joist Depth | 12" O.C. | 16" O.C. | 19.2" O.C. | 24" O.C. | 12" O.C. | 16" O.C. | 19.2" O.C. | 24" O.C. |
|  | 11-7/8" | 26' - 4' | 23'-11" | 22'-7" | 21'-0" | 28'-8' | 26'-1" | 24'-6" | 22' - 9" |
| CPI-90 | 16" | 33' - 0' | 30' - 1' | 28'-4' | 26'-4' | 36'-0" | 32'-9" | 30'-10" | 26' - 7" |

Highlighted sizes indicates stocked depths.

## Notes:

1. Table values apply to uniformly loaded CPI joists.

Use sizing software to analyze conditions outside of the scope of this table such as commercial floors, cantilevers or concealed loads.
2. Span is measured from face to face of supports. Use beam sizing software to analyze multiple span CPI joists if the length of any span is less than half the length of an adjacent span.
3. Live Load deflection is limited to L/480.
4. Table values assume sheathing is glued and nailed to the CPI joists. Reduce spans by 12 " if sheathing is nailed only.
5. Table values are based on $1-3 / 4^{\prime \prime}$ end and $3-1 / 2^{\prime \prime}$ intermediate bearing lengths without web stiffeners.

## NORDIC® ${ }^{\circledR}$-JOIST <br> UNIFORM LOADS

| Joist <br> Depth | Joist <br> Series | Criteria | Clear span (ft) |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | 8 | 10 | 12 | 14 | 16 | 18 | 20 | 22 | 24 | 26 | 28 | 30 |
| 9-1/2" | NI-40x | Live load (L/480) | - | - | 116 | 76 | 52 | 37 | 28 | 21 | 16 | 13 | 10 | - |
|  |  | Total load (L/240 | 233 | 187 | 155 | 114 | 88 | 69 | 56 | 42 | 33 | 26 | 21 | 17 |
|  | NI-80 | Live load (L/480) | - | - | - | 108 | 75 | 54 | 40 | 30 | 24 | 19 | 15 | 12 |
|  |  | Total load (L/240 | 233 | 187 | 157 | 135 | 118 | 105 | 81 | 61 | 48 | 38 | 30 | 25 |
| 11-7/8" | NI-40x | Live load (L/480) | - | - | 189 | 125 | 87 | 62 | 46 | 35 | 27 | 22 | 17 | 14 |
|  |  | Total load (L/240 | 288 | 231 | 193 | 148 | 114 | 90 | 73 | 60 | 51 | 43 | 35 | 29 |
|  | NI-60 | Live load (L/480) | - | - | - | 132 | 96 | 66 | 49 | 37 | 29 | 23 | 18 | 15 |
|  |  | Total load (L/240 | 288 | 231 | 193 | 166 | 146 | 118 | 96 | 75 | 59 | 46 | 37 | 30 |
|  | NI-80 | Live load (L/480) | - | - | - | - | 122 | 88 | 66 | 51 | 39 | 31 | 25 | 21 |
|  |  | Total load (L/240 | 288 | 231 | 193 | 166 | 146 | 129 | 117 | 102 | 79 | 63 | 51 | 42 |
|  | NI-90 | Live load (L/480) | - | - | - | 187 | 132 | 96 | 72 | 55 | 43 | 34 | 28 | 23 |
|  |  | Total load (L/240 | 326 | 262 | 219 | 188 | 165 | 147 | 132 | 111 | 87 | 69 | 56 | 46 |
| 14" | NI-60 | Live load (L/480) | - | - | - | - | 132 | 96 | 71 | 54 | 42 | 34 | 27 | 22 |
|  |  | Total load (L/240 | 305 | 245 | 205 | 176 | 154 | 137 | 116 | 96 | 81 | 68 | 55 | 45 |
|  | $\mathrm{NI}-80$ | Live load (L/480) | - | - | - | - | - | 126 | 95 | 73 | 57 | 45 | 37 | 30 |
|  |  | Total load (L/240 | 324 | 260 | 218 | 187 | 164 | 146 | 131 | 119 | 109 | 91 | 74 | 61 |
|  | NI-90 | Live load (L/480) | - | - | - | - | - | 136 | 102 | 79 | 62 | 49 | 40 | 33 |
|  |  | Total load (L/240 | 326 | 262 | 219 | 188 | 165 | 147 | 132 | 120 | 110 | 99 | 80 | 66 |
| 16" | NI-60 | Live load (L/480) | - | - | - | - | - | 128 | 96 | 74 | 57 | 46 | 37 | 30 |
|  |  | Total load (L/240 | 317 | 255 | 213 | 183 | 161 | 143 | 129 | 111 | 94 | 80 | 69 | 60 |
|  | $\mathrm{NI}-80$ | Live load (L/480) | - | - | - | - | - | - | 126 | 97 | 76 | 61 | 49 | 41 |
|  |  | Total load (L/240 | 354 | 284 | 238 | 204 | 179 | 159 | 144 | 131 | 120 | 111 | 97 | 82 |
|  | NI-90 | Live load (L/480) | - | - | - | - | - | - | 135 | 105 | 82 | 66 | 53 | 44 |
|  |  | Total load (L/240 | 354 | 284 | 238 | 204 | 179 | 159 | 144 | 131 | 120 | 111 | 103 | 88 |

## Notes:

1. The tabulated values may be used for simple or multiple spans.
2. For multiple-span applications, the end spans shall be $40 \%$ or more of the adjacent span.
3. I-joist shall satisfy both live load and total load. Where the live load is blank, the total load governs the design.
4. I-joist shall be laterally supported at points of bearing and along all compression edges.
5. Minimum bearing length shall be $1-3 / 4$ inch for end bearings and $3-1 / 2$ inches for intermediate bearings.
6. Bearing stiffeners are not required when l-joist are used in accordance with this table, excepts as required for hangers.
7. The tabulated values take into account a live load deflection limit of $L / 480$ and a total load deflection limit $L / 240$. Final design shall include a complete analysis including the verification of the bending moment and shear capacities.
8. For a live load deflection limit of $L / 360$, multiply live load values by 1.33
9. For double joists, double the table values and nail joist together per detail 1 p .

## I-JOIST

## FLOOR FRAMING \& CONSTRUCTION DETAILS

## COMMON CPI/N I JOIST FLOOR FRAMING

## AND CONSTRUCTION DETAILS

Some framing requirements such as erection bracing and blocking panels have been omitted for clarity.


## WEB STIFFENER REQUIREMENTS

Web stiffeners are pairs of small blocks, typically cut from wood structural panels, that are nailed to the joist web to stiffen a deep web, increase reaction capacity or accommodate a special connector. Web stiffeners are not required when joists are sized by means of the tables included in this guide, with the following exceptions:

1. Web stiffeners are required at the ends of joists set in hangers that are not deep enough to laterally support the top flanges of the joists. Refer to the hanger manufacturer's installation instructions.
2. Web stiffeners are required to accommodate special connector nailing requirements. Refer to the connector manufacturer's installation instructions.
3. Web stiffeners are required at birdsmouth cuts at the low end supports of sloped joists.
4. Web stiffeners are required at all supports on 22 and 24 inch joists.

When joists are sized by means of sizing software,
or otherwise engineered for an application, web stiffeners are required as follows:

1. Web stiffeners are required for high reactions at supports. Refer to ICC-ES ESR-1225.
2. Web stiffeners are required under concentrated loads applied to the tops of joists between supports, or along cantilevers beyond the support, when the concentrated load exceeds 1500 pounds.

## FIGURE $B$

WEBB STIFFENER REQUIREMENTS

| NUMBER OF WEB STIFFENER NAILS REQUIRED |  |  |  |
| :--- | :---: | :---: | :---: |
| Joist Depth | $24 " \& 22 "$ | $20 " \& 18 "$ | $16 " \&$ less |
| Intermediate Support | 10 | 8 | 4 |
| All Other Conditions | 8 | 6 | 4 |

## WEB STIFFENER SIZE REQUIRED

| Series | Flange <br> Width | Minimum Dimensions |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  | Web Stiffeners |  | Nails |
|  |  | Thickness | Width |  |
| N I-40X | 2-1/2" | $1{ }^{\prime \prime}$ | 2-5/16" | 2-1/2" $\times 0.131^{\prime \prime}$ |
| N I-60 | 2-1/2" | $1{ }^{\prime \prime}$ | 2-1/2" | 2-1/2" $\times 0.131^{\prime \prime}$ |
| N I-80 | 3-1/2" | 1-1/2" | 3-1/2" | $3-1 / 2^{\prime \prime} \times 0.131^{\prime \prime}$ |
| NI/CPI-90 | 3-1/2" | 1-1/2" | 3-1/2" | $3-1 / 2^{\prime \prime} \times 0.131^{\prime \prime}$ |

Web stiffener length is approximately $1 / 8$ " less than the clear distance between flanges.


## I－JOIST

FLOOR FRAMING \＆DETAILS

## TYPICAL CPI／NI JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS

All nails shown in the details below are assumed to be common nails unless otherwise noted．10d box nails may be substituted for 8d common shown in details．Individual components not shown to scale for clarity．

Provide lateral
bracing per 1a，1b， $\qquad$

Vertical load transfer per pair of squash blocks as shown：

| Pair of Squash Blocks | （lb） |
| :--- | :---: |
| $2 \times 4$ | 4000 |
| $1-1 / 8$＂Rim Board | 3000 |
| 1 Rim Board | 2700 |

VERTICAL LOAD CAPACITY

| Product（depths＝16＂） | Thickness | Vertical Load Capacity |
| :--- | :---: | :---: |
| Rim Joist／Blocking Panel | $3 / 8^{\prime \prime}$ Web | 2000 plf |
|  | $7 / 16^{\prime \prime}$ Web | 2850 plf |
| APA Rim Board | $1-1 / 8^{\prime \prime}$ | 4400 plf |

## I-JOIST

## FLOOR FRAMING \& DETAILS

1 g
Load bearing wall above shall align vertically with the wall below. Other conditions such as offset walls are not covered by this detail.


Backer block (use if hanger load exceeds 250 lbs .) Before installing a backer block to a double CPI/NI joist, drive 3 additional 10 d nails through the webs and filler block where the backer block will fit. Clinch.
install backer tight to top flange. Use twelve 10d nails, clinched when through the webs and filler block where the backer block will fit. Clinch
install backer tight to top flange. Use twelve 10 d nails, clinched when possible. Maximum capacity for hanger for this detail $=1280 \mathrm{lb}$.

## BACKER BLOCKS

(Blocks must be long enough to permit required nailing without splitting.)

| Flange Width | Material Thickness <br> Required* <br> $19 / 32^{\prime \prime}$ | Minimum Depth** |
| :---: | :---: | :---: |
| $1-1 / 2^{\prime \prime}$ | $23 / 32^{\prime \prime}$ | $5-1 / 2^{\prime \prime}$ |
| $1-3 / 4^{\prime \prime}$ | $1 "$ | $5-1 / 2^{\prime \prime}$ |
| $2-5 / 16^{\prime \prime}$ | $1 "$ | $7-1 / 4^{\prime \prime}$ |
| $2-1 / 2^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ | $5-1 / 2$ |
| $3-1 / 2^{\prime \prime}$ | $7-1 / 4^{\prime \prime}$ |  |

* Minimum grade for backer block material shall be Utility grade SPF (south) or better for solid sawn lumber and Rated Sheathing grade for wood structural panels.
** For face-mount hangers use net CPI/NI joist depth minus 3-1/4" for joists with 1-1/2" thick flanges. For $1-5 / 16^{\prime \prime}$ thick flanges use depth minus 2-7/8".


CPI/NI blocking panel
(Blocks must be long enough to permit required nailing without spiting.)

## CPI/NI BLOCKING PANELS

CPI/NI blocking panels prevent CPI/NI floor joists from overturning and transfer loads through the floor system into the structure below.

Due to difference in depth and possible shrinkage, common framing lumber set on edge is unacceptable as blocking. CPI/NI blocking panels must be cut to the proper length to between the CPI/NI joists, and their depth must match the depth of the joists.
CPI/NI blocking panels may be used:

1. To stabilize CPI/NI joists laterally at supports, as shown in Figures 1 a and 1 g . Lateral support is required during installation and is necessary to obtain design carrying capacity.
2. To transmit vertical loads up to 2,000 pf per CPI/NI blocking panel in accordance with Figures $1 \mathrm{a}, 1 \mathrm{c}$, 1 ff , and 1 g .

For hanger capacity see hanger manufacturer's recommendations. Verify double CPI/NI joist capacity to support concentrated loads.


Note: Unless hanger sides laterally support the top flange, web stiffeners shall be used. (See Figure B on page 7)

## Notes:

## I-JOIST

## CANTILEVER DETAILS

CPI/NI JOIST CANTILEVER DETAIL FOR INTERIOR BALCONIES


CANTILEVER DETAIL FOR VERTICAL BUILDING OFFSET

## CANTILEVER DETAIL FOR VERTICAL BUILDING OFFSET

Alternate Method 2
DOUBLE CPI/NI JOIST


Note: APA RATED SHEATHING 48/24 (minimum thickness $23 / 32$ ") required on sides of I-joist. Depth shall match the full height of the joist. Nail with 8d nails at 6" o.c., top and bottom flange. Install with face grain horizontal. Attach joist to plate at all supports per Detail 1b.


CPI/NI blocking panel or rim board blocking. Attach per Detail 1g.

Block CPI/NI joists together with filler blocks for the full length of the reinforcement. For joist flange widths greater than 3 ", place an additional row of 10 d nails along the centerline of the reinforcing panel from each side. Clinch when possible.

Face nail two rows 10d at 12" o.c. each side through one I-joist web and the filler block to other I-joist web. Offset nails from opposite face by 6". Clinch if possible (four nails per foot required, except two nails per foot required if clinched).

## WEB HOLES AND OPENINGS

## WEB HOLES IN I-JOISTS - Rules for Cutting Holes in I-Joists

1. The distance between the inside edge of the support and the centerline of any hole shall be in compliance with the requirements of table 6.1.
2. I-joist top and bottom flanges must never be cut, notched or otherwise modified.
3. Whenever possible, field-cut holes should be centered on the middle of the web.
4. The maximum size hole that can be cut into an I-joist web shall equal the clear distance between the flanges of the 1 -joist minus $1 / 4$ inch. A minimum of $1 / 8$ inch should always be maintained between the tip or bottom of the hole and the adjacent I-joist flange.
5. The sides of square holes or longest sides of rectangular holes should not exceed $3 / 4$ of the diameter of the maximum round hole permitted at that location.
6. Where more than one hole is necessary, the distance between adjacent hole edges shall exceed twice the diameter of the largest round hole or twice the size of the largest square hole -or twice the length of the longest side of the longest rectangular hole-and each hole must be sized and located in compliance with the requirements of table 6.1
7. Holes measuring 1-1/2 inch or smaller shall be permitted anywhere in a cantilevered section
 of a joist. Holes of greater size may be permitted subject to verification.
8. A $1-1 / 2$ inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above. For more than three holes per span, space holes at minimum 15 inches on center or contact Nordic Structures.
9. All holes shall be cut in accordance with the restrictions listed above and as illustrated in detail 6 a
10. Limit three maximum-size holes per span
11. A group of round holes at approximately the same location shall be permitted if it meets the requirements for a single round hole circumscribed around them.

| Joist <br> Depth | Joist Series | Round Hole Diameter (in.) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 2 | 3 | 4 | 5 | 6 | 6-1/4 | 7 | 8 | 8-5/8 | 9 | 10 | 10-3/4 | 11 | 12 | 12-3/4 |
| 9-1/2" | NI-40X | 0'-7" | 1'-4" | 2'-8" | 4'-2" | 5'-8" | 6'-2' |  |  |  |  |  |  |  |  |  |
|  | NI-80 | 2'-0" | 3'-5" | 4'-10" | 6'-4" | 8'-0" | 8'-5" |  |  |  |  |  |  |  |  |  |
| 11-7/8" | NI40X | 0'-7" | 0'-8" | 1'-0" | 2'-4" | 3'-8" | 4'-0" | 5'-2" | 6'-8" | 8'-0" |  |  |  |  |  |  |
|  | NI-60 | 0'-7" | 1'-4" | 2'-8" | 4'-0' | 5'-5" | 5'-10" | 7'-0" | 8'-8" | 9'-9" |  |  |  |  |  |  |
|  | NI-80 | 1'-4" | 2'-8" | 4'-0" | 5'-4" | 6'-10" | 7'-3" | 8'-5" | 10'-2" | 11'-3" |  |  |  |  |  |  |
|  | NI-90 | 0'-7" | 0'-8" | 1'-3" | 2'-11' | 4'-8" | 5'-2" | 6'-6" | 8'-6" | 9'-11" |  |  |  |  |  |  |
| 14" | NI-60 | 0'-7" | 0'-8" | 1'-3" | 2'-6" | 4'-0" | 4'-3" | 5'-3" | 6'-9" | 7'-9" | 8'-3' | 10'-2" | 11'-10" |  |  |  |
|  | NI-80 | 0'-8" | 1'-10" | 3'-2' | 4'-6" | 6'-0' | 6'-3" | 7'-4" | 8'-10" | 9'-10" | 10'-6" | 12'-3" | 13'-8" |  |  |  |
|  | NI-90 | 0'-7" | 0'-8' | 0'-9" | 2'-3' | 3'-10" | 4'-3" | 5'-6" | 7'-3' | 8'-5" | 9'-2' | 11'-2" | 12'-9" |  |  |  |
| 16" | NI-60 | 0'-7" | 0'-8' | 0'-8' | 1'-2" | 2'-5" | 2'-9" | 3'-9" | 5'-0" | 6'-0" | 6'-6" | 8'-0" | 9'-2" | 9'-8" | 11'-9" | 13'-9" |
|  | NI-80 | 0'-7" | 1'-2" | 2'-4" | 3'-8" | 5'-0" | 5'-4" | 6'-4" | 7'-10' | 8'-9" | 9'-4" | 11'-0" | 12'-2" | 12'-6" | 14'-4" | 16'-0" |
|  | NI-90 | 0'-7" | 0'-8" | 0'-8' | 1'-6" | $3^{\prime}-0{ }^{\prime \prime}$ | 3'-5" | 4'-6" | 6'-3" | 7'-3" | 7'-10" | 9'-8" | 11'-0" | 11'-6" | 13'-6" | 15'-3" |

## Notes:

1. Never drill, cut or notch the flange, or over-cut the web.
2. Holes in web should be cut with a sharp saw.
3. For rectangular holes, avoid over-cutting the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners is
recommended. Starting the rectangular hole by drilling a 1 -inch-diameter hole in each of the four corners and then making the cuts between the holes is another good method to minimize damage to the I-joist.

## DUCT CHASE OPENINGS - Rules for Cutting Duct Chase Opening in I-Joists

1. The distance between the inside edge of the support and the centerline of any hole shall be in compliance with the requirements of table 6.2.
2. I-joist top and bottom flanges must never be cut, notched or otherwise modified.
3. The maximum depth of a duct chase opening that can be cut into an l-joist web shall equal the clear distance between the flanges of the 1 -joist minus $1 / 4$ inch. A minimum of $1 / 8$ inch should always be maintained between the top or bottom of the opening and the adjacent l-joist flange.
4. All openings shall be cut in accordance with the restrictions listed above and as illustrated in detail 6b.
5. Limit one maximum-size duct chase opening per span.

## Notes:

1. Never drill, cut or notch the flange, or over-cut the web.
2. Holes in web should be cut with a sharp saw.
3. Avoid over-cutting the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners i recommended. Starting the rectangular hole by drilling a 1 inch diameter hole in each of the four corners and then making the cuts between the holes is another good method to minimize damage to the 1 -joist.

| Joist Depth | Joist <br> Series | Round Hole Diameter (in.) |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 8 | 10 | 12 | 14 | 16 | 18 | 20 | 22 | 24 |
| 9-1/2" | NI-40X | 5'-2" | 5'-7" | 6'-0" | 6'-4" | 6'-8" | 7'-2" | 7'-7" |  |  |
|  | NI-80 | 5'-2" | 5'-7" | 6'-0" | 6'-4" | 6'-8" | 7'-2" | 7'-7" | 8'-1" | 8'-6" |
| 11-7/8" | NI40X | 6'-7" | 7'-1" | 7'-6" | 8'-1" | 8'-6" | 9'-1" | 9'-7" |  |  |
|  | NI-60 | 7'-1' | 7'-7' | 8'-0" | 8'-4" | 8'-10" | 9'-3" | 9'-9" |  |  |
|  | NI-80 | 7'-1' | 7'-5" | 8'-0" | 8'-4" | 8'-10" | 9'-2" | 9'-8" | 10'-2" | 10'-8" |
|  | NI-90 | 4'-3" | 4'-10" | 5'-4" | 5'-11" | 6'-6" | 7'-1 | 7'-8" | 8'-3" | 8'-11' |
| 14" | NI-60 | 8'-8" | 9'-2' | 9'-6" | 10'-1" | 10'-6" | 11'-1" | 11'-7" |  |  |
|  | NI-80 | 8'-9" | 9'2 | 9'-8" | 10'-1' | 10'-6" | 11'-1" | 11'-6" | 12'-1" | 12'-8" |
|  | NI-90 | 5'-10" | 6'-5" | 7'-0" | 7'-6" | 8'-2' | 8'-9" | 9'-4" | 9'-11" | 10'-8" |
| 16" | NI-60 | 10'-1" | 10'-7" | 11'-0" | 11'-6" | 12'-1" | 12'-7" | 13'-4" |  |  |
|  | NI-80 | 10'-3" | 10'-9" | 11'-2" | 11'-7" | 12'-1" | 12'-7" | 13'-2" | 13'-9" | 14'-6" |
|  | NI-90 | 7'-4" | 7'-11" | 8'-6" | $9^{\prime}-1{ }^{\prime \prime}$ | 9'-8" | 10'-3" | 13'-0" | 11'-7" | 12'-3" |

## I-JOIST

ROOF FRAMING \& CONSTRUCTION DETAILS

COMMON CPI/N I-JOIST ROOF FRAMING AND CONSTRUCTION DETAILS


Optional overhangs


TYPICAL CPI/N I JOIST ROOF FRAMING AND CONSTRUCTION DETAILS
Individual components not shown to scale for clarity.

2a RIDGE CONNECTION - 12/12 MAXIMUM SLOPE

(2c) CPI/N I JOIST ABOVE CP-LAM RIDGE BEAM


Uplift connections may be required.

2b UPPER END, BEARING ON WALL


Uplift connections may be required.

2e CPI/N I JOIST ON BEVELED PLATE


2f BIRDSMOUTH CUT - LOW END OF CPI/N I JOIST ONLY



Uplift connections may be required.

[^0]
## I－JOIST <br> ROOF DETAILS

## COMMON CPI／N I－JOIST ROOF FRAMING AND CONSTRUCTION DETAILS

Individual components not shown to scale for clarity．

2 k optional overhang extensions for UNIFORMLY DISTRIBUTED LOADS ONLY

May be used with details 2d，2e and $2 f$（Low end only）
Stop CPI／N I Joist at wall line and extend top flange with $2 \times 4$ ．
Support extension with $2 \times 4$ nailed to web of joist with（2） rows of 8 d nails at 8 ＂o．c．clinched．Extend $2 \times 4$ support at least 4＇into joist span and nail to top flange with 8d nails at 8＂o．c．


Uplift connections may be required

## 2h BEVELED CUT BEARING STIFFENER



Uplift connections may be required


## I-JOIST

ALLOWABLE ROOF SPANS
Snow Load = 50 psf , Dead Load = 15 psf

| Joist Depth | Joist Series | Slope of 1/4: 12 to 4:12 On Centerspacing |  |  | Slope >4:12 to 8:12 On Centerspacing |  |  | Slope >8:12 to 12:12 On Centerspacing |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 12" | 16" | 24" | 12" | 16" | 24" | 12" | 16" | 24" |
| 9-1/2" | NI-40X | 21'-7" | 18'-8" | 15'-3" | 20'-6" | 18'-4" | 14'-11" | 19'-1" | 17'-3" | 14'-6" |
|  | NI-80 | 24'-8" | 22'-4" | 19'-4" | 23'-5" | 21'-2' | 18'-4" | 21'-9" | 19'-9" | 17'-1" |
| 11-7/8" | NI-40X | 24'-8" | 21'-4" | 17'-4" | 24'-2" | 20'-11" | 17'-0" | 22'-10" | 20'-4" | 16'-7" |
|  | $\mathrm{NI}-60$ | 26'-6" | 24'-0" | 19'-11" | 25'-1" | 22"-8" | 19'-6" | 23'-4" | 21'-2" | 18'-4" |
|  | $\mathrm{NI}-80$ | 29'-6" | 26'-8" | 23'-2" | 27'-11" | 25'-3" | 21'-11" | 26'-0" | 23'-7" | 20'-5" |
|  | NI-90 | 32'-5" | 27'-6" | 23'-10" | 28'-9" | 26'-1" | 22'-7" | 26'-10" | 24'-4" | 21'-1" |
| 14" | NI-60 | 30'-2" | 26'-10" | 21'-11' | 28'-7" | 25'-11" | 21'-6" | 26'-8" | 24'-1" | 20'-11" |
|  | NI-80 | 33'-7" | 30'-4" | 26'-1" | 31'-9" | 28'-9" | 24'-11" | 29'-7" | 26'-10" | 23'-3" |
|  | NI-90 | 34'-7" | 31'-3" | 27'-1" | 32'-8" | 29'-7" | 25'-8" | 30'-6" | 27'-7" | 24'-0" |
| 16" | NI-60 | 33'-6" | 28'-11" | 23'-7" | 31'-9" | 28'-5" | 23'-2" | 29'-7" | 26'-10" | 22'-6" |
|  | NI-80 | 37'-3" | 33'-8" | 28'-1" | 35'-3" | 31'-11" | 27'-7" | 32'-10" | 29'-9" | 25'-10" |
|  | NI-90 | 38'-8" | 34'-8" | 30'-1" | 36'-3" | 32'-10" | 28'-6" | 33'-9" | 30'-7" | 26'-7" |

## NOTES:

1. Allowable clear span applicable to simple-span roof construction with a design roof snow load as shown and dead load of 15 psf . The allowable span is based on the horizontal distance between inside face of supports. The snow load deflection is limited to L/240 and the total load deflection to L/180. Spans are based on a duration of load (DOL) factor of 1.15 .
2. Spans include a cantilever of up to 2 feet on one end of the I-joist.
3. Minimum bearing length shall be $1-3 / 4$ " inches for the end bearings, and $3-1 / 2^{\prime \prime}$ inches on end bearing adjacent to cantilever.
4. Bearing stiffeners are not required when I-joists are used with the spans and spacings given in these tables, except as required for hangers.
5. These span charts are based on uniform loads.

For applications with other than uniformly distributed
loads, an engineering analysis may be required based on the use of the design properties.

SI units conversion: 1 inch $=25.4 \mathrm{~mm}, 1 \mathrm{foot}=0.305 \mathrm{~m}$

CPI PRO JOISTS 50 PSF LIVE LOAD-15 PSF DEAD LOAD

| $\begin{gathered} \text { CPI } \\ \text { Joist Series } \end{gathered}$ | CPI <br> Joist Depth | Slope of 4/12 or Less |  |  | Slopes over 4/12 up to 8/12 |  |  | Slopes over 8/12 up to 12/12 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 16"O.C. | 19.2"O.C. | 24"0.C. | 16"O.C. | 19.2"O.C. | 24" O.C. | 16"O.C. | 19.2"O.C. | 24"O.C. |
| CPI 90 | 11-7/8" | 26'-10" | 25'-2" | 23'-2" | 25'-7" | 24'-0" | 22'-2" | 23'-11" | 22'-5" | 20'-9" |
|  | $14{ }^{\prime \prime}$ | 30'-5" | 28'-7" | 23'-2" | 29'-0" | 27'-3" | 22'-5" | 27'-2" | 25'-6" | 21'-5" |
|  | $16^{\prime \prime}$ | 33'-9" | 29'-1" | 23'-2" | 32'-2" | 28'-1" | 22'-5" | 30'-1" | 26'-10" | 21'-5" |

Stocked Joists


NOTES:

1. Table values apply to uniformly loaded simple or multiple span CPI joists. Span is the horizontal distance from face to face of supports. Use beam sizing software to analyze multiple span joists if the length of any span is less than half the length of an adjacent span.
2. Roofs must be sloped at least $1 / 4^{\prime \prime}$ in 12 " to assure drainage.
3. Live load deflection is limited to $L / 240$. Total load deflection is limited to $L / 180$. Verify that the deflection criteria conform to local building code requirements.
4. Table values are based on $1-3 / 4^{\prime \prime}$ end and $3-1 / 2^{\prime \prime}$ intermediate bearing lengths without web stiffeners.

## THE OPEN JOIST

The Barrette ${ }^{\otimes}$ Structural Open Concept Floor System
The strength of triangulation, accuracy of finger-jointed assembly, maximization of dimensional lumber and environmentally-friendly field adjustability makes Open Joist TRIFORCE ${ }^{\oplus}$ the only trimmable, all wood, open-webbed, finger-jointed floor joist installed without metal plate connectors.

Re-engineering wood components for your needs For more than 25 years, our finger joint technology has demonstrated its strength and durability throughout North America. The open joist TRIFORCE ${ }^{\circledR}$ has surpassed industry standards by establishing a new level of excellence in the engineering of floor systems, while optimizing the use of lumber in its components.


18 " up to 20 " available for order


## OPEN JOIST <br> FLOOR SPANS

## Maximum Allowable Spans

Glued \& Nailed Subfloor
$\Delta \mathrm{L} \leq \mathrm{L} / 480$

| Live Load |  |  |  | 40 psf |  |  |  | 40 psf |  |  |  | 100 psf |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Dead Load |  |  |  | 15 psf |  |  |  | 25 psf |  |  |  | 15 psf |  |  |  |
| Spacing |  |  |  | 12" | 16" | 19.2" | 24" | 12" | 16" | 19.2" | 24" | 12" | 16" | 19.2" | 24" |
|  | Sub | oor ${ }^{(7)}$ |  | 19/32" |  |  | 23/32" | 19/32" |  |  | 23/32" | 19/32" |  |  | 23/32" |
| Depth | Length | Series |  | Maximum Floor span o.c. |  |  |  |  |  |  |  |  |  |  |  |
| 11 7/8" | 8'-0" | OJ314 | $3 \times 2$ | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" |
|  | 10'-0" |  |  | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" |
|  | 12'-0" |  |  | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 11'-5" | 10'-3" |
|  | 14'-0" |  |  | 14'-0" | 14'-0" | 14'-0" | 14'-0" | 14'-0" | 14'-0" | 14'-0" | 13'-7" | 14'-0" | 12'-7" | ----- | ----- |
|  | 16'-0" |  |  | 16'-0" | 16'-0" | 16'-0" | 14'-10" | 16'-0" | 16'-0" | 15'-3" | ----- | 14'-3" | ----- | ----- | ----- |
|  | 18'-0" | OJ315 | $3 \times 2$ | 18'-0" | 18'-0" | 17'-6" | 16'-4" | 18'-0" | 18'-0" | 16'-9" | ----- | ----- | ----- | -- | ----- |
|  | 20'-0" | OJ415 | $4 \times 2$ | 20'-0" | 20'-0" | 19'-5" | 18'-2" | 20'-0" | 20'-0" | 19'-5" | -- | --- | ----- | ----- | ----- |
|  | 22'-0" | OJ418 | $4 \times 2$ | 22'-0" | 21'-9" | 20'-6" | - | 22'-0" | 21-9" | 20'-6" | ----- | ----- | ---- | ----- | -- |
| 14" | 8'-0" | OJ314 | $3 \times 2$ | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" |
|  | 10'-0" |  |  | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" |
|  | 12'-0" |  |  | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 11'-2" |
|  | 14'-0" |  |  | 14'-0" | 14'-0" | 14'-0" | 14'-0" | 14'-0" | 14'-0" | 14'-0" | 14'-0" | 14'-0" | 13'-9" | 12'-6" | ----- |
|  | 16'-0" |  |  | 16'-0" | 16'-0" | 16'-0" | 16'-0" | 16'-0" | 16'-0" | 16'-0" | 14'-11" | 16'-0" | --- | ----- | --- |
|  | 18'-0" | OJ315 | $3 \times 2$ | 18'-0" | 18'-0" | 18'-0" | 18'-0" | 18'-0" | 18'-0" | 18'-0" | 16'-5" | 16'-7" | -- | ----- | --- |
|  | 20'-0" | OJ418S ${ }^{(9)}$ | $4 \times 2$ | 20'-0" | 20'-0" | 20'-0" | 20'-0" | 20'-0" | 20'-0" | 20'-0" | 20'-0" | 19'-6" | --- | --- | ----- |
|  | 22'-0" | OJ415 | $4 \times 2$ | 22'-0" | 22'-0" | 22'-0" | 20'-7" | 22'-0" | 22'-0" | 22'-0' | -- | ----- | --- | ----- | ----- |
|  | 24'-0" | OJ418 | $4 \times 2$ | 24'-0" | 24'-0" | 23'-3" | ----- | 24'-0" | 24'-0" | 23'-3" | ----- | ----- | ----- | ----- | ----- |
|  | 26'-0" |  |  | 26'-0" | 24'-9" | ----- | ----- | 26'-0" | 24'-9" | ----- | -- | -- | ----- | --- | ----- |



## Maximum Allowable Spans

Glued \& Nailed Subfloor

$\Delta \mathrm{L} \leq \mathrm{L} / 480$

| Live Load |  |  |  | 40 psf |  |  |  | 40 psf |  |  |  | 100 psf |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Dead Load |  |  |  | 15 psf |  |  |  | 25 psf |  |  |  | 15 psf |  |  |  |
| Spacing |  |  |  | 12" | $16^{\prime \prime}$ | 19.2" | 24" | 12" | 16" | 19.2" | 24" | 12" | 16" | 19.2" | 24" |
| Subfloor ${ }^{(7)}$ |  |  |  | 19/32" |  |  | 23/32" | 19/32" |  |  | 23/32" | 19/32" |  |  | 23/32" |
| Depth | Length | Series |  | Maximum Floor span o.c. |  |  |  |  |  |  |  |  |  |  |  |
| 16" | 8'-0" | OJ314 | $3 \times 2$ | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" | 8'-0" |
|  | 10'-0" |  |  | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" | 10'-0" |
|  | 12'-0" |  |  | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0" | 12'-0' | 11'-8" |
|  | 14'-0" |  |  | 14'-0" | 14'-0" | 14'-0" | 14'-0" | 14'-0" | 14'-0" | 14'-0" | 14'-0" | 14'-0" | 14'-0" | 13'-4" | ----- |
|  | 16'-0' |  |  | 16'-0" | 16'-0" | 16'-0" | 16'-0" | 16'-0" | 16'-0" | 16'-0" | 16'-0" | 16'-0" | 14'-8" | ----- | ----- |
|  | 18'-0" | OJ315 | $3 \times 2$ | 18'-0" | 18'-0" | 18'-0" | 18'-0" | 18'-0" | 18'-0" | 18'-0" | 17'-9" | 18'-0" | 16'-4" | ----- | ----- |
|  | 20'-0" | OJ315 | $3 \times 2$ | 20'-0" | 20'-0" | 20'-0" | 19'-3" | 20'-0" | 20'-0" | 19'-10" | ----- | 18'-4" | ----- | ----- | --- |
|  | 22'-0" | OJ418 | 4×2 | 22'-0" | 22'-0" | 22'-0" | 22'-0" | 22'-0" | 22'-0" | 22'-0" | 20'-9" | $\underline{21-7 "}$ | ---- | ---- | --- |
|  | 24'-0" |  |  | 24'-0" | 24'-0" | 24'-0" | 24'-0" | 24'-0" | 24'-0" | 24'-0" | --- | -- | -- | ----- | -- |
|  | 26'-0" |  |  | 26'-0" | 26'-0" | 25'-8" | ----- | 26'-0" | 26'-0" | 25'-8" | ----- | --- | ----- | --- | -- |
|  | 28'-0" | OJ420 | $4 \times 2$ | 28-0" | 28'-0" | 26'-6" | ---- | 28'-0" | 28'-0" | 25'-11" | --- | -- | ----- | ----- | --- |
|  | 30'-0" |  |  | 30'-0" | 28'-2" | ----- | ----- | 30'-0" | 28'-2" | ----- | ----- | ----- | ----- | ----- | --- |

## Notes:

1. The indicated spans are based on simple span joists.
2. Minimum end bearing length is $11 / 2^{\prime \prime}$, spans values in bold indicate that web stiffeners are required at the OSB end panel.
3. Maximum spans are measured centerline to centerline of bearing and are based on uniformly loaded joists.
4. Total Load deflection is limited to $\mathrm{L} / 240$.
5. Live Load deflection is limited to $L / 480$.
6. The indicated spans are based on the Allowable Stress Design method as per NDS, ICC IBC/IRC code.
7. The considered subfloor is a 20 " o.c. APA rated panel for joist spacing of $12^{\prime \prime}, 16^{\prime \prime}$ and $19.2^{\prime \prime}$ o.c. and is a 24 " o.c. APA rated panel for joist spacing of $24^{\prime \prime}$ o.c. The subfloor must be glued as per APA Specification AFG-01 or ASTM D3498 and nailed as per NDS, ICC IBC/IRC code.
8. Refer to appropriate sections of the Specifier Guide for installation guidelines and construction details.
9. $\mathrm{S}=$ Limited inventory. Please contact your representative to determine quantities.

## FIRE PERFORMANCE CERTIFICATIONS

TRIFORCE ${ }^{\oplus}$ open joist has been certified for equivalent fire performance to $2 \times 10$ nominal dimension lumber, qualifying as an exception to 2018/2021 IRC fire performance requirements for an unfinished basement ceiling (Section R302.13 - Exception 4). See detail below.

Moreover, TRIFORCE ${ }^{\oplus}$ open joists are rigorously tested by INTERTEK, and independent third-party offering Total Quality Assurance to industries worldwide (SpecID 35685). With certified durations of 45, 60, 90 and 120 minutes, assemblies have been engineered and are available through our fire-resistant solutions brochure.


## OPEN JOIST <br> FLOORDETAILS

## Mechanical Clearances

| Mechanical Opening Dimension |  |  |  |
| :---: | :---: | :---: | :---: |
| Depth | Round | Square | Rectangular |
| 11-7/8' | $71 / 4 "$ | $53 / 4 " \times 53 / 4 "$ | 3" $\times 13$ " |
| $14^{\prime \prime}$ | $81 / 2^{\prime \prime}$ | $61 / 22^{\prime \prime} \times 61 / 2^{\prime \prime}$ | $3^{\prime \prime} \times 14$ ", 6" $\times 8$ " |
| $16^{\prime \prime}$ | $91 / 2^{\prime \prime}$ | $7112{ }^{\prime \prime} \times 71 / 2^{\prime \prime}$ | 3" $\times 15$ " |

Possibility of round hole at $41 / 2^{\prime \prime}$ o.c. of joist extremity. Contact your TRIFORCE ${ }^{\oplus}$ representative for more details.

## Typical Details



## STRONGBACKS <br> DETAILS

Strongbacks must be dry lumber and secured with 2 spiral or resined 3" nails or 2-3" screws at mid-span, to a vertical brace or diagonal web.

Strongbacks can be cut between 2 joists for ducts, pipes and wires if needed, but at least 3 consecutive joists must remain attached together.
$11-7 / 8^{\prime \prime}=2 \times 4$ " or $2 \times 6$ "
$14^{\prime \prime}=2 \times 6$ " or $2 \times 8^{\prime \prime}$
$16^{\prime \prime}=2 \times 6$ ", $2 \times 8$ " or $2 \times 10 "$


## Detail 5

Strongback (at mid span)

## Option \#1 (recommanded)



Secure vertical side block ( $2 \times 4$ ) as per detail, with 2 nails* to both chords and strongback to vertical with 2 nails*. *(gun nails $0.122^{\prime \prime} \times 3$ ¹⁄4")

Adding adhesive will provide an ultimate connection for high floor performance. Gun nails can be substituted with 3" screws.

## Option \#2

2x3 flanges: $1-3^{\prime \prime}$ (10d) through bottom flange and $1-3^{\prime \prime}$ (10d) through the diagonal, adding adhesive will insure long term performance


2x4 flanges: $2-3^{\prime \prime}$ (10d) through bottom flange and 1-3" (10d) through the diagonal.

Adding adhesive will ensure long term performance. Gun nails can be substituted with 3 " screws.

## Strongback Overlap



## SIMPSON

Strongylie
${ }^{\circledR}$
JOIST HANGER CHART

| I-JOIST SIZE | TOP MOUNT | FACE MOUNT | TOP MOUNT DOUBLE | FACE MOUNT DOUBLE |
| :---: | :---: | :---: | :---: | :---: |
| NI-40 9-1/2" | ITS25695 | IUS25695 | MIT3952 | MIU5129 |
| NI-40 11-7/8" | ITS2561188 | IUS2561188 | MIT311882 | MIU51211 |
| NI-60 11-7/8" | ITS2561188 | IUS2561188 | MIT311882 | MIU51211 |
| NI-60 14" | ITS25614 | IUS25614 | MIT3142 | MIU51211 |
| NI-60 16" | ITS35616 | IUS35616 | MIT3162 | MIU51216 |
| NI-80 9-1/2" | ITS35695 | IUS35695 | BA712195 | HU410-2 |
| NI-80 11-7/8" | ITS3561188 | IUS3561188 | BA7121188 | HU412-2 |
| NI-80 14" | ITS35614 | IUS35614 | BA71214 | HU414-2 |
| NI-80 16" | ITS35616 | IUS35616 | BA71216 | HU414-2 |
| NI-90 11-7/8" | ITS3561188 | IUS3561188 | BA7121188 | HU410-2 |
| NI-90 14" | ITS35614 | IUS35614 | BA71214 | HU412-2 |
| NI-90 16" | ITS35616 | IUS35616 | BA71216 | HU414-2 |
| CPI-90 11-7/8" | ITS3561188 | IUS3561188 | BA7121188 | HU410-2 |
| CPI-90 14" | ITS35614 | IUS35614 | BA71214 | HU412-2 |
| CPI-90 16" | ITS35616 | IUS35616 | BA71216 | HU414-2 |
| OPEN JOIST <br> TRI-FORCE | TOP MOUNT | FACE MOUNT | TOP MOUNT DOUBLE | FACE MOUNT DOUBLE |
| $11-7 / 8^{\prime \prime} \times 3^{\prime}-18^{\prime}$ | ITS2561188 | IUS2561188 | MIT311882 | M1451211 |
| $11-7 / 8$ " $\times 201-22$ | ITS3561188 | IUS3561188 | BA7121188 | HU412-2 |
| $14^{\prime \prime} \times 3{ }^{\prime}-18^{\prime}$ | ITS25614 | IUS25614 | MIT3142 | MI451211 |
| $14^{\prime \prime} \times 19^{\prime}-24^{\prime}$ | ITS35616 | IUS35614 | BA71214 | HU414-2 |
| $16^{\prime \prime} \times 3{ }^{\prime}-16^{\prime}$ | ITS25616 | IUS25616 | MIT3182 | M1451214 |
| $16^{\prime \prime} \times 18^{\prime}-30^{\prime}$ | ITS35616 | IUS35616 | BA71216 | HU414-2 |

All items in stock
For a complete list of all stocked Simpson Connectors, email us at ewp@coastal.com

Engineered Rim Board is a structural framing member designed to support wall loads and tie floor joists together.

Engineered Rim Board must be continuously supported along the bottom edge and not used to span openings. It may not be used as other structural framing elements such as joists, rafters, headers and ledgers.

## ADVANTAGES

## - No delamination

- Manufactured to match the depths of I-joist framing members
- Resistant to moisture
- Dimensionally stable
- 12 foot standard


## PERFORMANCE CRITERIA

Norbord Rim Board is manufactured in accordance with ICBO AC-124 Acceptance Criteria for Wood-Based Rim Board Products.

## STORAGE AND HANDLING

Ship Rim Board under tarp. Set bundles on supports to keep Rim Board off the ground and provide air circulation. Outdoors, keep Rim Board under a protective cover. When high moisture exists, cut banding on the stack to prevent edge damage. When using a forklift, put the stack on a pallet or supports to minimize damage from forks.


ALLOWABLE DESIGN PROPERTIES - 1 3/4" 2.1E CP-LAM

1. These allowable design stresses apply to dry service conditions.
2. No increase is allowed for load duration.
3. Multiply by $(12 / \mathrm{d})^{1 / 5}$ where $\mathrm{d}=$ depth of member (in).

4. A factor of 1.04 may be applied for repetitive members as defined in the National Design Specification for Wood Construction

FOR ADDITIONAL GRADES AND SIZES PLEASE VISIT OUR WEBSITE AT WWW.COASTALFP.COM

### 2.1ECP - LAM FLOOR BEAMS

This table provides CP-LAM beam sizes for center support of one level of floor framing over various column spacings. Where floor joists are continuous over the beam, this table applies only when the 'A' span is between $45 \%$ and $55 \%$ of the building width.


| Width of Building (ft) | Column Spacing |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 11' | 12' | 13' | 14' | 15' | 16' | 17' | 18' | 19' | 20' |
| 24' | 2-11-7/8" | 2-11-7/8" | 2-11-7/8" | 2-14" | 2-14" | 2-16" | 2-16" | 2-16" | 2-18" | 2-18" |
|  | 3-9-1/2" | 3-9-1/2" | 3-11-7/8" | 3-11-7/8" | 3-11-7/8" |  |  |  |  |  |
| 28' | 2-11-7/8" | 2-11-7/8" | 2-14" | 2-14" | 2-14" | 2-16" | 2-16" | 2-18" | 2-18" | - |
|  | 3-9-1/2" | 3-11-7/8" | 3-11-7/8" | 3-11-7/8" | 3-14" | 3-14" | 3-14" | 3-16" | 3-16" | 3-18" |
| 32' | 2-11-7/8" | 2-11-7/8" | 2-14" | 2-14" | 2-16" | 2-16" | 2-18" | 2-18" | - | - |
|  | 3-9-1/2" | 3-11-7/8" | 3-11-7/8" | 3-14" | 3-14" | 3-14" | 3-16" | 3-16" | 3-18" | 3-18" |
| 36' | 2-11-7/8" | 2-14" | 2-14" | 2-16" | 2-16" | 2-18" | 2-18" | - | - | - |
|  | 3-11-7/8" | 3-11-7/8" | 3-11-7/8" | 3-14" | 3-14" | 3-16" | 3-16" | 3-16" | 3-18" | 3-18" |
| 40' | 2-11-7/8" | 2-14" | 2-14" | 2-16" | 2-16" | 2-18" | - | - | - | - |
|  | 3-11-7/8" | 3-11-7/8" | 3-14" | 3-14" | 3-14" | 3-16" | $3-16{ }^{\prime \prime}$ | 3-18" | 3-18" | - |

## Notes:

1. CP-LAM beam sizes are listed as the number of $1-3 / 4$ " thick pieces by the beam depth, e.g. 2-1/2 indicates two 1-3/4" pieces by 9-1/2" deep.
2. All CP-LAM beams require support across their full width.
3. The minimum required end and intermediate bearing lengths (based on 850 psi ) are 3 " and 7-1/2" respectively unless the + symbol is shown. In that case, 4-1/2" and 10-1/2" end and intermediate bearing lengths are required.
4. CP-LAM beam sizes are based on residential floor loading of 40 psf live load and 10 psf dead load. The roof framing must be trusses supported at the exterior walls only.
5. Defection is limited to $L / 360$ at live load and $L / 240$ at total load.
6. CP-LAM beam sizes are based on continuous floor joist spans and simple or continuous beam spans. If the floor joists are not continuous, it is permissible to consider a "Width of Building" dimension that is equal to 0.8 times the actual width of the building.

### 2.1E CP-LAM

## ALLOWABLE UNIFORM LOADS FLOOR 100\%

ALLOWABLE UNIFORM LOADS* - POUNDS PER LINEAR FOOT-1-3/4" 2.1E CP-LAM

|  | Key | One 1-3/4"CP-LAM |  |  | Two 1-3/4"CP-LAM |  |  |  |  | Three 1-3/4"CP-LAM |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 9-1/2" | 11-7/8" | $14^{\prime \prime}$ | 9-1/2" | 11-7/8" | 14" | $16^{\prime \prime}$ | 18" | 9-1/2" | 11-7/8" | $14^{\prime \prime}$ | $16^{\prime \prime}$ | 18" |
| 6 | LL | - | - | - | - | - | - | - | - | - | - | - | - | - |
|  | TL | 1063 | 1425 | 1796 | 2127 | 2850 | 3591 | 4388 | 5304 | 3190 | 4275 | 5387 | 6582 | 7955 |
|  | BRG | 2.2/5.4 | 2.9/7.2 | 3.6/9.1 | 2.2/5.4 | 2.9/7.2 | 3.6/9.1 | 4.4/11.1 | 5.4/13.4 | 2.2/5.4 | 2.9/7.2 | 3.6/9.1 | 4.4/11.1 | 5.4/13/4 |
| 8 | LL | 724 | - | - | 1447 | - | - | - | - | 2171 | - | - | - | - |
|  | TL | 746 | 979 | 1208 | 1493 | 1958 | 2416 | 2887 | 3404 | 2239 | 2937 | 3624 | 4331 | 5105 |
|  | BRG | 2/5 | 2.6/6.6 | 3.3/8.2 | 2/5 | 2.6/6.6 | 3.3/8.2 | 3.9/8.8 | 4.6/11.5 | 2/5 | 2.6/6.6 | 3.3/8.2 | 3.9/9.8 | 4.6/11.5 |
| 10 | LL | 370 | 724 |  | 741 | 1447 | - |  | - | 1111 | 2171 |  |  |  |
|  | TL | 551 | 745 | 909 | 1103 | 1490 | 1819 | 2150 | 2504 | 1654 | 2236 | 2728 | 3224 | 3755 |
|  | BRG | 1.9/4.7 | 2.5/6.3 | 3.1/7.7 | 1.9/4.7 | 2.5/6.3 | 3.1/7.7 | 3.6/9.1 | 4.2/10.6 | 1.9/4.7 | 2.5/6.3 | 3.1/7.7 | 3.6/9.1 | 4.2/10.6 |
| 11 | LL | 278 | 544 | - | 557 | 1087 | - | - | - | 835 | 1631 | - | - | - |
|  | TL | 413 | 665 | 809 | 826 | 1331 | 1618 | 1905 | 2211 | 1240 | 1996 | 2427 | 2858 | 3316 |
|  | BRG | 1.5/3.9 | 2.5/6.2 | 3/7.5 | 1.5/3.9 | 2.5/6.2 | 3/7.5 | 3.5/8.9 | 4.1/10.3 | 1.5/3.9 | 2.5/6.2 | 3/7.5 | 3.5/8.9 | 4.1/10.3 |
| 12 | LL | 214 | 419 | 686 | 429 | 837 | 1372 | - | - | 643 | 1256 | 2058 | - | - |
|  | TL | 317 | 586 | 729 | 635 | 1172 | 1452 | 1711 | 1979 | 952 | 1758 | 2186 | 2566 | 2968 |
|  | BRG | 1.5/3.2 | 2.4/6 | 3/7.4 | 1.5/3.2 | 2.4/6 | 3/7.4 | 3.5/8.7 | 4/10.1 | 1.5/3.2 | 2.4/6 | 3/7.4 | 3.5/8.7 | 4/10.1 |
| 13 | LL | 169 | 329 | 540 | 337 | 659 | 1079 | - | - | 506 | 988 | 1619 | - | - |
|  | TL | 249 | 489 | 663 | 497 | 977 | 1325 | 1552 | 1790 | 746 | 1466 | 1988 | 2328 | 2686 |
|  | BRG | 1.5/3 | 2.2/5.4 | 2./9/7.3 | 1.5/3 | 2.2/5.4 | 2.9/7.3 | 3.4/8.6 | 3.9/9.9 | 1.5/3 | 2.2/5.4 | 2.9/7.3 | 3.4/8.6 | 3.9/9.9 |
| 14 | LL | 135 | 264 | 432 | 270 | 527 | 864 | 1290 | - | 405 | 791 | 1296 | 1935 | - |
|  | TL | 198 | 390 | 578 | 396 | 780 | 1156 | 1420 | 1635 | 595 | 1170 | 1734 | 2130 | 2452 |
|  | BRG | 1.5/3 | 1.9/4.7 | 2.8/6.9 | 1.5/3 | 1,9/4.7 | 2.8/6.9 | 3.4/8.4 | 3.9/9.7 | 1.5/3 | 19/4.7 | 2.8/6.9 | 3.4/8.4 | 3.9/9.7 |
| 15 | LL | 110 | 214 | 351 | 220 | 429 | 703 | 1049 | 1493 | 329 | 643 | 1054 | 1573 | 2240 |
|  | TL | 160 | 316 | 503 | 321 | 632 | 1006 | 1280 | 1504 | 481 | 949 | 1508 | 1921 | 2255 |
|  | BRG | 1.5/3 | 1.6/4.1 | 2.6/6.4 | 1.5/3 | 1.6/4.1 | 2.6/6.4 | 3.3/8.2 | 3.8/9.6 | 1.5/3 | 1.6/4.1 | 2.6/6.4 | 3.3/8.2 | 3.8/9.6 |
| 16 | LL | 90 | 177 | 289 | 181 | 353 | 579 | 864 | 1230 | 271 | 530 | 868 | 1296 | 1846 |
|  | TL | 131 | 260 | 428 | 263 | 519 | 856 | 1124 | 1391 | 394 | 779 | 1284 | 1685 | 2086 |
|  | BRG | 1.5/3 | 1.5/3.6 | 2.3/5.8 | 1.5/3 | 1.5/3.6 | 2.3/5.8 | 3.1/7.7 | 3.8/9.5 | 1.5/3 | 1.5/3.6 | 2.3/5.8 | 3.1/7.7 | 3.8/9.5 |
| 17 | LL | 75 | 147 | 241 | 151 | 295 | 483 | 720 | 1026 | 226 | 442 | 724 | 1081 | 1539 |
|  | TL | 109 | 216 | 356 | 218 | 431 | 711 | 994 | 1230 | 326 | 647 | 1067 | 1490 | 1845 |
|  | BRG | 1.5/3 | 1.5/3.2 | 2.1/5.2 | 1.5/3 | 1.5/3.2 | 2.1/5.2 | 2.9/7.2 | 3.6/8.9 | 1.5/3 | 1.5/3.2 | 2.1/5.2 | 2.9/7.2 | 3.6/8.9 |
| 18 | LL | 64 | 124 | 203 | 127 | 248 | 407 | 607 | 864 | 191 | 372 | 610 | 910 | 1296 |
|  | TL | 91 | 181 | 299 | 182 | 361 | 597 | 885 | 1095 | 273 | 542 | 896 | 1327 | 1643 |
|  | BRG | 1.5/3 | 1.5/3 | 1.8/4.6 | 1.5/3 | 1.5/3 | 1.8/4.6 | 2.7/6.8 | 3.4/8.4 | 1.5/3 | 1.5/3 | 1.8/4.6 | 2.7/6.8 | 3.4/8.4 |
| 19 | LL | 54 | 105 | 173 | 108 | 211 | 346 | 516 | 735 | 162 | 316 | 519 | 774 | 1102 |
|  | TL | 77 | 153 | 253 | 153 | 306 | 506 | 760 | 981 | 230 | 459 | 759 | 1139 | 1472 |
|  | BRG | 1.5/3 | 1.5/3 | 1.7/4.1 | 1.5/3 | 1.5/3 | 1.7/4.1 | 2.5/6.2 | 3.2/8 | 1.5/3 | 1.5/3 | 1.7/4.1 | 2.5/6.2 | 3.2/8 |
| 20 | LL | 46 | 90 | 148 | 93 | 181 | 296 | 442 | 630 | 139 | 271 | 445 | 664 | 945 |
|  | TL | 65 | 130 | 216 | 130 | 261 | 432 | 649 | 884 | 195 | 391 | 648 | 974 | 1326 |
|  | BRG | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3.7 | 2.2/5.6 | 3/7.6 | 1.5/3 | 1.5/3 | 1.5/3.7 | 2.2/5.6 | 3/7.6 |
| 22 | LL | 35 | 68 | 111 | 70 | 136 | 223 | 332 | 473 | 104 | 204 | 334 | 499 | 710 |
|  | TL | 48 | 97 | 161 | 96 | 193 | 321 | 484 | 694 | 144 | 290 | 482 | 726 | 1040 |
|  | BRG | 1.5/3 | 1.5/3 | 1.5/3.1 | 1.5/3 | 1.5/3 | 1.5/3.1 | 1.8/4.6 | 2.6/6.6 | 1.5/3 | 1.5/3 | 1.5/3.1 | 1.8/4.6 | 2.6/6.6 |
| 24 | LL | 27 | 52 | 86 | 54 | 105 | 172 | 256 | 365 | 80 | 157 | 257 | 384 | 547 |
|  | TL | 36 | 73 | 122 | 72 | 146 | 245 | 370 | 530 | 108 | 219 | 367 | 554 | 796 |
|  | BRG | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3.9 | 2.2/5.5 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3.9 | 2.2/5.5 |
| 26 | LL | 21 | 41 | 67 | 42 | 82 | 135 | 201 | 287 | 63 | 124 | 202 | 302 | 430 |
|  | TL | 27 | 56 | 95 | 55 | 113 | 190 | 288 | 414 | 82 | 169 | 284 | 431 | 621 |
|  | BRG | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3.3 | 1.9/4.7 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3.3 | 1.9/4.7 |
| 28 | LL | 17 | 33 | 54 | 34 | 66 | 108 | 161 | 230 | 51 | 99 | 162 | 242 | 344 |
|  | TL | 21 | 44 | 75 | 42 | 88 | 149 | 227 | 328 | 63 | 132 | 224 | 341 | 492 |
|  | BRG | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.6/4.1 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.6/4.1 |
| 30 | LL | 14 | 27 | 44 | 27 | 54 | 88 | 131 | 187 | 41 | 80 | 132 | 197 | 280 |
|  | TL | 16 | 35 | 60 | 33 | 70 | 119 | 182 | 264 | 49 | 104 | 179 | 273 | 395 |
|  | BRG | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3.5 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3.5 |

## Can be applied to the CP-LAM beam in addition to its own weight. - Simple or multiple CP-LAM beam spans

## Key to Table:

LL = Maximum live load- limits deflection to L/360
TL= Maximum total load - limits deflection to L/240
$B R G=$ Required end/intermediate bearing length (inches), based on plate bearing stress of 850 psi.

### 2.1E CP-LAM

ALLOWABLE UNIFORM LOADS ROOF SNOW 115\%
ALLOWABLE UNIFORM LOADS* - POUNDS PER LINEAR FOOT-1-3/4" 2.1E CP-LAM

| Span (ft) | Key | One 1-3/4" CP-LAM |  |  | Two 1-3/4" CP-LAM |  |  |  |  | Three 1-3/4" CP-LAM |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 9-1/2" | 11-7/8" | 14" | 9-1/2" | 11-7/8" | 14 " | 16" | 18" | 9-1/2" | 11-7/8" | $14^{\prime \prime}$ | 16" | 18" |
| 6 | TL | 1224 | 1640 | 2006 | 2447 | 3279 | 4132 | 5049 | 6102 | 3671 | 4919 | 6198 | 7573 | 9152 |
|  | LL | - | - | - | - | - | - | - | - | - | - | - | - | - |
|  | BRG | 2.5/6.2 | 3.3/8.3 | 4.2/10.4 | 2.5/6.2 | 3.3/8.3 | 4.2/10.4 | 5.1/12.8 | 6.2/15.4 | 2.5/6.2 | 3.3/8.3 | 4.2/10.4 | 5.1/12.8 | 6.2/15.4 |
| 8 | TL | 859 | 1127 | 1390 | 1718 | 2254 | 2780 | 3323 | 3917 | 2577 | 3380 | 4170 | 4984 | 5875 |
|  | LL | - | - | - | - | - | - | - | - | - | - | - | - | - |
|  | BRG | 2.3/5.8 | 3/7.6 | 3.8/9.4 | 2.3/5.8 | 3/7.6 | 3.8/9.4 | 4.5/11.2 | 5.3/13.2 | 2.3/5.8 | 3/7.6 | 3.8/9.4 | 4.5/11.2 | 5.3/13.2 |
| 10 | LL | 556 | - | - | 1111 | - | - | - | - | 1667 | - | - | - | - |
|  | TL | 651 | 858 | 1047 | 1302 | 1716 | 2093 | 2474 | 2882 | 1954 | 2573 | 3140 | 3711 | 4322 |
|  | BRG | 2.2/5.5 | 2.9/7.3 | 3.5/8.8 | 2.2/5.5 | 2.8/7.3 | 3.5/8.8 | 4.2/10.5 | 4.9/12.2 | 2.2/5.5 | 2.9/7.3 | 3.5/8.8 | 4.2/10.5 | 4.9/12.2 |
| 11 | LL | 418 | - | - | 835 | - | - | - | - | 1253 | - | - | - | - |
|  | TL | 537 | 766 | 931 | 1075 | 1532 | 1863 | 2193 | 2545 | 1612 | 2298 | 2794 | 3290 | 3817 |
|  | BRG | $2 / 5$ | 2.9/7.1 | 3.5/8.7 | $2 / 5$ | 2.9/7.1 | 3.5/8.7 | 4.1/10.2 | 4.7/11.8 | $2 / 5$ | 2.9/7.1 | 3.5/8.7 | 4.1/10.2 | 4.7/11.8 |
| 12 | LL | 322 | 628 | - | 643 | 1256 | - | - | - | 965 | 1884 | - | - |  |
|  | TL | 424 | 675 | 839 | 849 | 1350 | 1678 | 1970 | 2278 | 1273 | 2025 | 2517 | 2954 | 3417 |
|  | BRG | 1.7/4.3 | 2.7/6.9 | 3.4/8.5 | 1.7/4.3 | 2.7/6.9 | 3.4/8.5 | 4/10 | 4.6/11.6 | 1.7/4.3 | 2.7/6.9 | 3.4/8.5 | 4/10 | 4.6/11.6 |
| 13 | LL | 253 | 494 | - | 506 | 988 | - | - | - | 759 | 1482 | - | - |  |
|  | TL | 333 | 574 | 763 | 666 | 1148 | 1526 | 1787 | 2061 | 999 | 1723 | 2289 | 2681 | 3092 |
|  | BRG | 1.5/3.7 | 2.5/6.3 | 3.4/8.4 | 1.5/3.7 | 2.5/6.3 | 3.4/8.4 | 3.9/9.8 | 4.5/11.3 | 1.5/3.7 | 2.5/6.3 | 3.4/8.4 | 3.9/9.8 | 4.5/11.3 |
| 14 | LL | 203 | 396 | 648 | 405 | 791 | 1296 | - | - | 608 | 1187 | 1944 | - | - |
|  | TL | 266 | 494 | 666 | 531 | 989 | 1332 | 1635 | 1882 | 797 | 1483 | 1997 | 2453 | 2823 |
|  | BRG | 1.5/3.2 | 2.4/5.9 | 3.2/7.9 | 1.5/3.2 | 2.4/5.9 | 3.2/7.9 | 3.9/9.7 | 4.5/11.2 | 1.5/3.2 | 2.4/5.9 | 3.2/7.9 | 3.9/9.7 | 4.5/11.2 |
| 15 | LL | 165 | 322 | 527 | 329 | 643 | 1054 | - | - | 494 | 965 | 1581 | - | - |
|  | TL | 215 | 423 | 579 | 430 | 847 | 1158 | 1475 | 1732 | 646 | 1270 | 1737 | 2212 | 2597 |
|  | BRG | 1.5/3 | 2.2/5.4 | 30/7.4 | 1.5/3 | 2.2/5.4 | $3 / 7.4$ | 3.8/9.4 | 4.4/11 | 1.5/3 | 2.2/5.4 | $3 / 7.4$ | 3.8/9.4 | 4.4/11 |
| 16 | LL | 136 | 265 | 434 | 271 | 530 | 868 | - | - | 407 | 795 | 1303 | - | - |
|  | TL | 177 | 348 | 508 | 353 | 696 | 1016 | 1294 | 1602 | 530 | 1044 | 1525 | 1941 | 2402 |
|  | BRG | 1.5/03 | 1.9/4.8 | 2.8/6.9 | 1.5/3 | 1.9/4.8 | 2.8/6.9 | 3.8/8.8 | 4.4/10.9 | 1.5/3 | 1.8/4.8 | 2.8/6.9 | 3.5/8.8 | 4.4/10.9 |
| 17 | LL | 113 | 221 | 362 | 226 | 442 | 724 | 1081 | - | 339 | 663 | 1086 | 1621 | - |
|  | TL | 146 | 289 | 449 | 293 | 578 | 899 | 1145 | 1417 | 439 | 867 | 1348 | 1717 | 2125 |
|  | BRG | 1.5/3 | 1.7/4.2 | 2.6/6.5 | 1.5/3 | 1.7/4.2 | 2.6/6.5 | 3.3/8.3 | 4.1/10.2 | 1.5/3 | 1.7/4.2 | 2.6/6.5 | 3.3/8.3 | 4.1/10.2 |
| 18 | LL | 95 | 186 | 305 | 191 | 372 | 610 | 910 | - | 286 | 558 | 915 | 1366 | - |
|  | TL | 123 | 243 | 400 | 245 | 485 | 800 | 1020 | 1262 | 368 | 728 | 1208 | 1529 | 1893 |
|  | BRG | 1.5/3 | 1.5/3.8 | 2.5/6.2 | 1.5/3 | 1.5/3.8 | 2.5/6.2 | 3.1/7.8 | 3.9/9.7 | 1.5/3 | 1.5/3.8 | 2.5/6.2 | 3.1/7.8 | 3.9/9.7 |
| 19 | LL | 81 | 158 | 259 | 162 | 316 | 519 | 774 | 1102 | 243 | 475 | 778 | 1161 | 1653 |
|  | TL | 104 | 206 | 339 | 207 | 411 | 679 | 914 | 1131 | 311 | 617 | 1018 | 1370 | 1696 |
|  | BRG | 1.5/3 | 1.5/3.4 | 2.2/5.5 | 1.5/3 | 1.5/3.4 | 2.2/5.5 | $3 / 7.4$ | 3.7/9.2 | 1.5/3 | 1.5/3.4 | 2.2/5.5 | $3 / 7.4$ | 3.7/9.2 |
| 20 | LL | 69 | 136 | 222 | 139 | 271 | 445 | 664 | 945 | 208 | 407 | 667 | 996 | 1418 |
|  | TL | 88 | 175 | 290 | 177 | 351 | 580 | 823 | 1019 | 265 | 526 | 870 | 1235 | 1529 |
|  | BRG | 1.5/3 | 1.5/3 | $2 / 5$ | 1.5/3 | 1.5/3 | $2 / 5$ | 2.8/7 | 3.5/8.7 | 1.5/3 | 1.5/3 | $2 / 5$ | 2.8/7 | 3.5/8.7 |
| 22 | LL | 52 | 102 | 167 | 104 | 204 | 334 | 499 | 710 | 157 | 306 | 501 | 748 | 1065 |
|  | TL | 65 | 131 | 216 | 131 | 261 | 433 | 650 | 839 | 196 | 392 | 649 | 975 | 1259 |
|  | BRG | 1.5/3 | 1.5/3 | 1.6/4.1 | 1.5/3 | 1.5/3 | 1.6/4.1 | 2.5/6.1 | 3.2/7.9 | 1.5/3 | 1.5/3 | 1.6/4.1 | 2.5/6.1 | 3.2/7.9 |
| 24 | LL | 40 | 79 | 129 | 80 | 157 | 257 | 384 | 547 | 121 | 236 | 386 | 576 | 820 |
|  | TL | 49 | 99 | 165 | 99 | 199 | 330 | 498 | 703 | 148 | 298 | 496 | 746 | 1054 |
|  | BRG | 1.5/3 | 1.5/3 | 1.5/3.5 | 1.5/3 | 1.5/3 | 1.5/3.5 | 2.1/5.2 | 2.9/7.3 | 1.5/3 | 1.5/3 | 1.5/3.5 | 2.1/5.2 | 2.9/7.3 |
| 26 | LL | 32 | 62 | 101 | 63 | 124 | 202 | 302 | 430 | 95 | 185 | 304 | 453 | 645 |
|  | TL | 38 | 77 | 129 | 76 | 154 | 257 | 388 | 557 | 114 | 231 | 386 | 582 | 836 |
|  | BRG | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.8/4.4 | 2.5/6.3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.8/4.4 | 2.5/6.3 |
| 28 | LL | 25 | 49 | 81 | 51 | 99 | 162 | 242 | 344 | 76 | 148 | 243 | 363 | 517 |
|  | TL | 29 | 61 | 102 | 59 | 121 | 203 | 308 | 443 | 88 | 182 | 305 | 462 | 664 |
|  | BRG | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3.8 | 2.2/5.4 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3.8 | 2.2/5.4 |
| 30 | LL | 21 | 40 | 66 | 41 | 80 | 132 | 197 | 280 | 62 | 121 | 198 | 295 | 420 |
|  | TL | 23 | 48 | 81 | 46 | 96 | 163 | 248 | 357 | 69 | 145 | 244 | 371 | 535 |
|  | BRG | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3.3 | 1.9/4.7 | 1.5/3 | 1.5/3 | 1.5/3 | 1.5/3 | 1.9/4.7 |

Can be applied to the CP-LAM beam in addition to its own weight. • Simple or multiple CP-LAM beam spans
Key to Table:
LL = Maximum live load- limits deflection to L/360
$T L=$ Maximum total load - limits deflection to $L / 240$
$B R G=$ Required end/intermediate bearing length (inches), based on plate bearing stress of 850 psi .

## BEARING DETAILS

BEARING ON EXTERIOR WALL
Prevent direct contact of CP-LAM with concrete. Consult local building code for requirements

BEAM-TO-BEAM CONNECTION
Make sure hanger capacity is appropriate for each application. Hangers must be properly installed to accommodate full capacity


## 3b

EARING ON WOOD COLUMN
Verify the required bearing area and the ability of the supporting column member to provide adequate strength


BEARING ON STEEL COLUMN
Verify the required bearing area and the ability of the supporting column member to provide adequate strength


WINDOW/DOOR
HEADER -
2-STORY
TYPICAL
See "Bearing
Length Requirements" below


## BEARING LENGTH REQUIREMENTS

## CP-LAM BEARING LENGTH REQUIREMENTS

| Support <br> Material |  | $\begin{gathered} \hline \text { S-P-F (South) } \\ \text { Hem-Fir } \\ \text { (North) }^{(5)} \\ \hline \end{gathered}$ |  | $\underset{\mathbf{S}-\mathrm{P}-\mathrm{F}^{(5)}}{\text { Hem-Fir }}$ |  | Southern Pine Douglas Fir-Larch ${ }^{(5)}$ |  | $\begin{gathered} \text { 2.1E } \\ \text { CP-LAM }{ }^{(6)} \end{gathered}$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathrm{F}_{\mathrm{CL}}$ (psi) |  |  |  | 405 |  | 565 |  | 850 |  |
| $\begin{array}{\|r\|} \hline \text { CP-LA } \\ \text { Wi } \end{array}$ |  | 1-3/4" | 3-1/2" | 1-3/4" | 3-1/2" | 1-3/4" | 3-1/2" | 1-3/4" | 3-1/2" |
|  | 1 | 3" | 1-1/2" | 1-1/2" | 1-1/2" | 1-1/2" | 1-1/2" | 1-1/2" | 1-1/2" |
|  | 2 | 3-1/2" | 3" | $3{ }^{\prime \prime}$ | 1-1/2" | 3" | 1-1/2" | 1-1/2" | 1-1/2" |
|  | 3 | 5-1/2" | 3" | 4-1/2" | $3 "$ | 3-1/2" | 3" | 3" | 1-1/2" |
|  | 4 | 7-1/2" | 3-1/2" | $6{ }^{\prime \prime}$ | 3" | 4-1/2" | 3" | 3" | 1-1/2" |
|  | 5 | 9-1/4" | 4-1/2" | 7-1/4" | 4-1/2" | 5-1/2" | 3" | 3-1/2" | 3" |
|  | 6 |  | 5-1/2" | 9-1/4" | 4-1/2" | 7-1/4" | 3-1/2" | 4-1/2" | $3{ }^{\prime \prime}$ |
|  | 7 |  | $6 "$ |  | 5-1/2" | 7-1/4" | 4-1/2" | 5-1/2" | 3" |
|  | 8 |  | 7-1/4" |  | $6 "$ | 9-1/4" | 4-1/2" | 5-1/2" | 3-1/2" |
|  | 9 |  | 9-1/4" |  | 7-1/4" | 9-1/4" | 5-1/2" | 7-1/2" | 3-1/2" |
|  | 10 |  | 9-1/4" |  | 7-1/4" |  | 5-1/2" | 7-1/2" | 3-1/2" |
|  | 11 |  |  |  | 9-1/4" |  | $6 "$ | 7-1/2" | 4-1/2" |
|  | 12 |  |  |  | 9-1/4" |  | 7-1/4" | 9" | 4-1/2" |


| Support Material |  | S-P-F (South) |  | Hem-FirS-P-F(5) |  | Southern Pine Douglas Fir-Larch ${ }^{(5)}$ |  | $\begin{gathered} \text { 2.1E } \\ \text { CP-LAM }{ }^{(6)} \end{gathered}$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathrm{F}_{\text {cL }}$ (psi) |  | 335 |  | 405 |  | 565 |  | 850 |  |
| $\begin{array}{r} \text { CP-LA } \\ \mathbf{W i} \end{array}$ | Beam <br> (in) | 1-3/4" | 3-1/2" | 1-3/4" | 3-1/2" | 1-3/4" | 3-1/2" | 1-3/4" | 3-1/2" |
|  | 13 |  |  |  | 9-1/4" |  | 7-1/4" |  | 4-1/2" |
|  | 14 |  |  |  |  |  | 7-1/4" | 9" | 5-1/2" |
|  | 15 |  |  |  |  |  | 9-1/4" |  | 5-1/2" |
|  | 16 |  |  |  |  |  | 9-1/4" |  | 5-1/2" |
|  | 17 |  |  |  |  |  | 9-1/4" |  | $6 "$ |
|  | 18 |  |  |  |  |  | 9-1/4" |  | 7-1/2" |
|  | 19 |  |  |  |  |  |  |  | 7-1/2" |
|  | 20 |  |  |  |  |  |  |  | 7-1/2" |
|  | 21 |  |  |  |  |  |  |  | 7-1/2" |
|  | 22 |  |  |  |  |  |  |  | 7-1/2" |
|  | 23 |  |  |  |  |  |  |  | 9" |

## Notes:

1. The minimum required bearing length is $1-1 / 2^{\prime \prime}$
2. Duration of load factors may not be applied to bearing length requirements.
3. All CP-Lam beams require support across their full width.
4. All CP-LAM beams require lateral support at bearing points.

## HOLE DETAILS

5. Use these values when the CP-LAM beam is supported by a wall plate, sill plate, timber or built up girder.
6. Use these values when the CP-LAM beam is supported by the end of a column or connection hardware.
7. The support member must be sized to carry the load from the CP-LAM beam.

## HOLES IN CP-LAM BEAMS <br> NOTES:



1. This technical note applies only to uniformly loaded, simple and multiple span CP-LAM beams. Beams that carry concentrated loads, or cantilevered beams, are outside the scope of this technical note.
2. Square and rectangle holes are not permitted.
3. Round holes may be drilled or cut with a hole saw anywhere within the shaded area of the CP-LAM beam.
4. The horizontal distance between adjacent holes must be at least two times the size of the larger hole. This restriction also applies to the location of access holes relative to bolt holes in multi-ply CP-LAM beams.
5. Do not drill more than three access holes in any four foot long section of CP-LAM beam.
6. The maximum round hole diameter permitted is:

| CP-LAM Beam Depth | $5-1 / 2^{\prime \prime}$ | $7-1 / 2^{\prime \prime}$ | $9-1 / 2^{\prime \prime}$ to $24{ }^{\prime \prime}$ |
| :--- | :--- | :--- | :--- |
| Maximum Hole Diameter | $3 / 4^{\prime \prime}$ | $1^{\prime \prime}$ | $1-1 / 2^{\prime \prime}$ |

7. These limitations apply to holes drilled for plumbing or wiring access only. The size and location of holes drilled for fasteners are governed by the provisions of National Design Specifications $®$ for wood construction.
8. CP-LAM beams deflect under load. Size holes to provide clearance where required.

COMBINATIONS OF $13 / 4$ " AND $3 ½$ " PLIES

## NAILS



Condition A
(2) $-1^{3 / 4}$

(3) - $13 / 1$ Condition B
(3) $-13 / 4$ or $\begin{array}{r}(1)-13 / 4 \\ +(1)-31 / 2\end{array}$


Condition C (2) $-1^{3 / 4}{ }^{\prime \prime}$
$+(1)-31 /{ }^{1}$

Condition D
(4) $-1^{3 / 4}$

$$
\text { Y Nail Spacing } \quad \uparrow
$$



133" AND $31 / 2$ " PLIES-MAXIMUM UNIFORM SIDE LOAD (PLF)

| Condition | 31/4" X 0.131" Nails |  | 16d Common Nails |  |
| :---: | :---: | :---: | :---: | :---: |
|  | $\begin{gathered} \text { 2Rows } \\ \text { at } 12^{20 .} \end{gathered}$ | $\begin{gathered} 3 \text { Rows } \\ \text { at 12" o.c. } \end{gathered}$ | $\begin{gathered} \hline 2 \text { Rows } \\ \text { at } 12^{\prime \prime} 0 . C . \end{gathered}$ | $\begin{gathered} 3 \text { Rows } \\ \text { at } 12^{\prime \prime} \mathrm{oc.c.} . \end{gathered}$ |
| Condition A (2-13/4) | 390 | 585 | 565 | 845 |
| Condition B ( $3-13 / 4$ " OR 1-13/4 $+1-31 / 2^{\prime \prime}$ ) | 290 | 435 | 425 | 635 |
| Condition ( ( $2-13 / 4{ }^{\prime \prime}+1-31 / 2{ }^{\prime \prime}$ ) | 260 | 390 | 375 | 565 |
| Condition D (4-13/4) | Use bolts for this condition (see note 1). |  |  |  |
| Condition E (2-31/2) | Use bolts for this condition (see note 1). |  |  |  |

## Notes:

1. Minimum fastener schedule for smaller side loads and top-loaded beams: Conditions $A, B \& C$, beams 12 " deep or less: 2 rows $3 / 3^{\prime \prime} \times 0.131$ "at 12 " o.c.
 Conditions D \& E, all beam depths: $\quad 2$ rows $1 / 2$ bolts at 24 " $0 . \mathrm{c}$.
2. The table values for nails may be doubled for 6 " 0. .. and tripled for 4 " 0. . nail spacings.
3. The nail schedules shown apply to both sides of a three-ply beam.
4. The table values apply to bolts meeting the requirements of ANSI/ASME Standard B18.2.1. A standard cut washer, or metal plate or strap of equal or greater dimensions, shall be provided between the wood and the bolt head and between the wood and the nut. The distance from the edge of the beam to the bolt holes must be at least 2 " for $1 / 2$ " bolts. Bolt holes shall be the same diameter as the bolt.
5. 7 " wide beams must be loaded from both sides and/or top loaded.
6. Beams wider than 7 " must be designed by the engineer of record.
7. Load duration factors may be applied to the table values.
8. For proprietary fastener alternatives, consult the manufacturer's literature.


## SIDELOADED 13/4 MULTI-PLY SCL ASSEMBLIES -

ALLOWABLE UNIFORM LOAD APPLIED TO EITHER OUTSIDE MEMBER

| Multiple Members |  | Nomina Screw Length (in) | Loaded side | Structural Composite Lumber |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $\begin{aligned} & \text { SDW @ } \\ & 12 " 0 . c . \end{aligned}$ |  | $\begin{aligned} & \text { SDW @ } \\ & 16 " 0 . c . \end{aligned}$ |  | $\begin{aligned} & \text { SDW @ } \\ & \text { 24" o.c. } \end{aligned}$ |  |
| Assembly | mponents |  |  | 2 Rows | 3 Rows | 2 Rows | 3 Rows | 2 Rows | Rows |
| A-W | 2-ply SCL |  | 33/8 | Either | 1600 | 2400 | 1200 | 1800 | 800 | 1200 |
| B-W | 3-ply SC | 5 | Head | 1200 | 1800 | 900 | 1350 | 600 | 900 |
|  |  |  | Tip | 900 | 1350 | 675 | 1015 | 450 | 675 |
| C-W | 4-ply S | 63/4 | Head | 1065 | 1600 | 800 | 1200 | 535 | 800 |
|  |  |  | Tip | 800 | 1200 | 600 | 900 | 400 | 600 |

1. Each ply is assumed to carry same proportion of load.
2. Loads may be applied to the head side and point side concurrently provided neither published allowable load is exceeded. Example: a 3-ply assembly with a head side load of 1300 plf and point side load of 1000 plf may be fastened together with 3 rows of SDW @ 16" o.c.)
3. When hangers are installed on point side, hanger face fasteners must be a minimum of 3 " long.
4. Tables are based on Main Member Penetration as noted in Single-Fastener Load Tables of the Simpson Strong-Tie Fastening Systems 2017-2018 Catalog C-F-2017 (page 358).
5. Please consult strongtie.com for the latest fastener details and data.

## Installation

- SDW screws install best with a lowspeed $1 / 2$ " drill and a $T$-40 6 -lobe bit. The matched bit included with the screws is recommended for best results.
- Screw heads that are countersunk flush to the wood surface are acceptable if the screw has not spun out.
- Individual screw locations may be adjusted up to 3 " to avoid conflicts with other hardware or to avoid lumber defects.

| SCREW DIMENSIONS |  |  |  |
| :--- | :---: | :---: | :---: |
| Model No. | Nominal <br> Screw <br> Lenth (L) <br> (in) | Thread <br> Lenth <br> (TL) (in) | Head <br> Stamp <br> Length |
| SDW22338 | $33 / 8$ | $19 / 16$ | 3.37 |
| SDW22500 | 5 | 1916 | 5.00 |
| SDW22634 | $63 / 4$ | $11 / 16$ | 6.75 |

- Pre-drilling is typically not required.


## How to Use the Maximum Uniform Side Load Table

EXAMPLE: THREE $13 / 4$ " PLIES LOADED FROM BOTH SIDES AND ABOVE (COND. B)

1. Use allowable load tables or sizing software to size the beam to carry a total load of $(300+610+550)=1460$ plf.
2. Refer to the Condition B row in the table. Scan across the row from left to right for a table value greater than 550 plf, which is the greatest side load carried by the beam. The fourth value in the row indicates that 3 rows of 16 d common nails at 12 " 0 .c. will accommodate a side load of 635 plf which is greater than the 550 plf required. Use 3 rows of 16d common nails at 12 " 0. .., from both sides, to assemble the beam.


## PWT TREATED LVL

## TREATED LAMINATED VENEER LUMBER

## REFERENCE DESIGN VALUE

## DRY USE

True (Shear-Free) Modulus of Elasticity, $\quad \mathrm{E}=2,000,000^{(1)(4)}$
Bending (beam), $\quad \mathrm{Fb}=2,800^{(2)(3)}$
Horizontal Shear (beam), $\quad \mathrm{Fv}=285 \mathrm{psi}$
Compression perpendicular to grain [psi], $\mathrm{FC}=850 \mathrm{psi}$
(1) Do not adjust for load duration.
(2) Adjust by ( $12 / \mathrm{d})^{0.2}$, where d is the depth of the member [inches].
(3) Adjust by 1.04 for repetitive members as defined in the NDS.
(4) True (Shear-Free) modulus of elasticity does not account for shear deformation.
(5) See APA Product Report PR-L329.

## NOTE:

Not all exterior conditions are wet-use and not all interior conditions are dry use. Flashing tape required to meet PWT
For more information on what determines wet use conditions, visit pacificwoodtech.com
joists, supported by PWT Treated LVL ledger or beams and columns, with proper connectors and fasteners.

PWT Treated LVL ledger or beams and columns per local code requirements, with proper deck anchors/ties, connectors, and fasteners.

## NOTES:

1. For diagonal bracing, see AWC Deck Construction Guide, page 10, figure 10 located at pacificwoodtech.com/treated.
2. For flashing tape recommendations, visit pacificwoodtech.com/treated.
3. For fastener and hanger information, visit strongtie.com/deckcenter.
4. Design conditions outside of the scope of this guide may be designed using CSD Software.

Treated right

## PWT TREATED LVL <br> TREATED LAMINATED VENEER LUMBER

PWT Treated LVL is treated throughout each layer therefore, "re-treatment" is unnecessary. However, all end cuts should be coated with a sealer or paint to minimize swelling, as moisture will wick into end-grain fibers more quickly than edges and faces.

Flashing or approved flashing tape is required for all upward facing horizontal surfaces. Coastal supplies Joist Guard by Henry or HydroFlash from Benjamin Obdyke. A complete list of approved tapes is available at pacificwoodtech.com

## Product Highlights

- PWT Treated LVL is the only manufacturer-treated LVL, and it is covered by a 25-year limited, transferable warranty.
- PWT Treated LVL is protected against damage caused by fungal rot, decay and wood-destroying insects, including Formosan termites (interior or exterior usage.)
- We use a proprietary treatment system and process, utilizing TRU-CORE ${ }^{\oplus}$ technology.


## The Product

- PWT Treated LVL may be used in exterior construction above-ground applications (UC3B) and for components that are difficult to maintain, repair, or replace and that are critical to the performance and safety of the entire system:
- Deck substructures, exterior columns, sill plates and fascia
- Treatment is added during the LVL manufacturing process, which fully penetrates throughout each veneer layer, offering complete protection from the inside out - No treatment gradient - and double (2X) the preservative retention required in various standards around the world
- Additionally, envelope treated for best surface properties

| 2.0 PWT Treated TVL Joist Span Table |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Dry Use- Maximum Allowable Deck Joist Spans Without Overhangs L/360 50 psf 10 psf |  |  |  |  |
| Species | Nominal Size | Joist Spacing (oc) |  |  |
| 2.0 true PWT <br> Treated TVL |  | $12^{\prime \prime}$ | $16^{\prime \prime}$ | 24 " |
|  | 13/4" x $91 / 2^{\prime \prime}$ | 19'01" | 17' $04{ }^{\prime \prime}$ | 15' 02 " |
|  | $13 / 4{ }^{\prime \prime} \times 117 / 8^{\prime \prime}$ | 23' 11" | 21'09" | 19'00" |
|  | $13 / 4$ " $14{ }^{\prime \prime}$ | 28' 02" | 25'07" | 22' 04" |
|  | $2 \times 8$ | 13' 06" | 12'03" | 10' 06" |
|  | 2x10 | 17' 02" | 15' 08" | 13' $05^{\prime \prime}$ |
|  | $2 \times 12$ | 20'11" | 19'00" | 16' 04" |



PWT

## COASTAL ENGINEERED FRAMING LUMBER

## Stocked at Coastal Forest Products Up to 32' Lengths

$2 \times 4$ up to 20<br>$2 \times 10$ (9-1/2') up to $32^{\prime}$<br>$2 \times 12$ (11-7/8") up to 32<br>$2 \times 8$ up to 28

1.6 MOE

- Same size as SPF/Fir (1-1/2")
- Low moisture content means dimensionally stable
- Ideal for long rafters (up to 32')
- Similar spans like I-joists
- Approved as substitute in new IRC fire code
- Uses standard size joist hangers
- Douglas Fir
- Frame roof with traditional compression ridge and collar ties
- No problem notching birdmouths
- Every piece is wane free!
- Excellent product for stair stringers!
- No cracks, rot or large knots, dried to 12\%
- Engineered sizing available through CSD
- I-Struct software
- MOE 1.6 (Modulas of Elasticity)
- Fb 2250 psi (Fiber Bending)
- Fv 230 psi (Shear)
- Fc 1600 psi (Compression Parallel to Grain)
- These values are based on normal load duration.
- When structural members qualify as repetitive members in accordance with applicable code, a 4\% increase is permitted to Fb.
- Manufactured by Pacific Woodtech



## ENGINEERED STUDS

## LAMINATED VENEER LUMBER

## LAMINATED VENEER LUMBER ENGINEERED FOR STRUCTURAL FRAMING

Extra-long PWLVL Dimension offers a stronger, stiffer, and straighter product than dimension lumber for all your structural applications. PWLVL Dimension is competitive in materials cost and is easy to handle and install, which can result in shorter construction schedules, saving you time and money. Build with confidence. Use beam-calculating software for better optimization of material selection and on-center spacing. PWLVL Dimension is available in virtually any length.

## PWLVL DIMENSION DESIGN PROPERTY COMPARISON ${ }^{(1)(2)}$

| Product |  | Modulus of Elasticity E (psi) | Bending $\mathrm{Fb}(\mathrm{psi})^{(3)}$ | Horizontal Shear $\mathrm{F}_{\mathrm{V}}$ (psi) | Compression Parallel to Grain $\mathrm{Fc}(\mathrm{psi})^{(4)}$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\underset{\sim}{\star}$ | 1.5" x 3.5" x 1.6E PWLVL | 1600000 | 2995 | 230 | 1950 |
|  | 2x4 Douglas Fir-Larch No. 2 | 1600000 | 1555 | 180 | 1550 |
|  | 2x4 Spruce-Pine-Fir No. 1 / No. 2 | 1400000 | 1510 | 135 | 1325 |
|  | 2x4 Hem-Fir No. 2 | 1300000 | 1465 | 150 | 1495 |
|  | 2x4 Western Woods No. 2 | 1000000 | 1165 | 135 | 1035 |
| $\underset{\sim}{\bullet}$ | 1.5" $\times 5.5$ " $\times 1.6 \mathrm{E}$ PWLVL | 1600000 | 2735 | 230 | 1950 |
|  | 2x6 Douglas Fir-Larch No. 2 | 1600000 | 1345 | 180 | 1485 |
|  | 2x6 Spruce-Pine-Fir No. 1 / No. 2 | 1400000 | 1310 | 135 | 1265 |
|  | 2x6 Hem-Fir No. 2 | 1300000 | 1270 | 150 | 1430 |
|  | 2x6 Western Woods No. 2 | 1000000 | 1010 | 135 | 990 |
| $\underset{\sim}{\infty}$ | $1.5{ }^{\prime \prime} \times 7.25$ " $\times 1.6 \mathrm{EPWLVL}$ | 1600000 | 2590 | 230 | 1950 |
|  | 2x8 Douglas Fir-Larch No. 2 | 1600000 | 1240 | 180 | 1420 |
|  | 2x8 Spruce-Pine-Fir No. 1 / No. 2 | 1400000 | 1205 | 135 | 1210 |
|  | 2x8 Hem-Fir No. 2 | 1300000 | 1175 | 150 | 1365 |
|  | 2x8 Western Woods No. 2 | 1000000 | 930 | 135 | 945 |
| $\underset{\sim}{\text { 글 }}$ | 1.5 " $\times 9.25$ " $\times 1.6 \mathrm{EPWLVL}$ | 1600000 | 2465 | 230 | 1950 |
|  | 2x10 Douglas Fir-Larch No. 2 | 1600000 | 1140 | 180 | 1350 |
|  | 2x10 Spruce-Pine-Fir No. 1 / No. 2 | 1400000 | 1105 | 135 | 1150 |
|  | 2x10 Hem-Fir No. 2 | 1300000 | 1075 | 150 | 1300 |
|  | 2x10 Southern Pine No. 2 | 1400000 | 920 | 175 | 1300 |
| $\underset{\sim}{\underset{\sim}{x}}$ | 1.5" x 11.25" x 1.6E PWLVL | 1600000 | 2370 | 230 | 1950 |
|  | 2x12 Douglas Fir-Larch No. 2 | 1600000 | 1035 | 180 | 1350 |
|  | 2x12 Spruce-Pine-Fir No. 1 / No. 2 | 1400000 | 1005 | 135 | 1150 |
|  | 2x12 Hem-Fir No. 2 | 1300000 | 975 | 150 | 1300 |
|  | 2x12 Southern Pine No. 2 | 1400000 | 860 | 175 | 1250 |

Notes:

1. Refer to APA PR-L233 for PWLVL adjustment factors and other design properties.
2. Refer to the 2015 NDS ${ }^{\circledR}$ for lumber adjustment factors and other design properties.
3. Load applied to the narrow face of the member. Repetitive member and size factors have been applied where applicable.
4. Size factors have been applied to lumber values where applicable.
5. MOE is a True (Shear-Free MOE) and it does not account for shear deformation.


## COASTALENGINEERED FRAMING LUMBER SPANS

## L/480 ALLOWABLE RESIDENTIAL FLOOR SPANS-40 PSF LIVE LOAD AND 15 PSF DEAD LOAD

| Chart Based Upon Uniform Loads |  | Simple Span |  |  | Multiple Span |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Size | Strength | 12" o.c. | 16" o.c. | 19.2" 0.c. | 12" o.c. | 16" o.c. | 19.2" o.c. |
| 11/2" $\times 91 / 2^{\prime \prime}$ | 1.6 MOE | 17'-5" | 16'-2" | 15'-4" | 19'6" | 18'-1" | 17'-1" |
| 1112" $\times 117 / 8^{\prime \prime}$ | 1.6 MOE | 21'-7" | 19'-11" | 18'-11" | 24'-2" | 22'-3" | 20'-0" |

## L/360 ALLOWABLE RESIDENTIAL FLOOR SPANS-40 PSF LIVE LOAD AND 15 PSF DEAD LOAD

| Chart Based Upon Uniform Loads |  | Simple Span |  |  | Multiple Span |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Size | Strength | 12" 0.c. | 16" o.c. | 19.2" 0.c. | 12" 0.c. | 16" 0.c. | 19.2" o.c. |
| $11 / 2^{\prime \prime} \times 91 / 2^{\prime \prime}$ | 1.6 MOE | 19'-3" | 17'-9" | 16'-11" | 21'-6" | 18'-9" | 17'-1" |
| $11 / 2{ }^{1} \times 117 /{ }^{\prime \prime}$ | 1.6 MOE | 23'-9" | 21'-11" | 20'-10" | 26-7" | 23'-0" | 20'-0" |

## Notes:

1. Table values apply to uniformly loaded, residential floor joists.

Highlighted columns indicate stocked sizes
2. Span is measured from face to face of supports.
3. Deflection is limited to $L / 240$ at total load and $L / 480$ or $L / 360$ at live load.
4. Table values are based on glued and nailed sheathing panels (19/32"). Use an ASTM D3498 adhesive in accordance with the manufacturer's recommendations.
5. Provide at least $1 \frac{1}{2} 2^{\prime \prime}$ of bearing length at end supports, $2^{\prime \prime}$ for spans in [brackets], and $31 / 2$ " at intermediate supports.
6. Provide lateral restraint at supports (e.g. full-depth solid blocking, rim board) and along the compression edge of each joist (e.g. floor sheathing).
7. Use sizing software or consult a professional engineer to analyze conditions outside the scope of this table (e.g. commercial floors, different bearing conditions, concentrated loads) or for multiple span joists if the length of any span is less than half the length of an adjacent span.
8. 14 " and 16 " $\mathrm{m} 1 / 3$ ultiple-span joists require full-depth, solid blocking at $1 / 3$-points along each span.
9. Table values are based on design properties adjusted to account for the Allowable Holes shown to the right.

## Allowable Holes:

1. Round holes only. Holes must be drilled with a bit or cut with a hole saw.
2. Maximum diameter $=1 / 2$ of the beam depth
3. Maximum 2 holes per span
4. Minimum clearance from edge of hole to:

- edge of adjacent hole -2 times the diameter of the larger hole
- edge of beam $-1 / 3$ of the beam depth - face of support - 6 inches


## RAFTERS: ROOF—SNOW 115\%

## ALLOWABLE RAFTER SPAN - L/360

| $\begin{aligned} & \text { Rafter } \\ & \text { Spacing } \\ & \text { (0.c.) } \end{aligned}$ | Rafter <br> Size [in.] |  | Roof Snow Load (PSF) |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $30 \mathrm{LL}+10 \mathrm{DL}$ |  |  | $40 \mathrm{LL}+10 \mathrm{DL}$ |  |  | $50 \mathrm{LL}+10 \mathrm{DL}$ |  |  |
|  |  |  | Roof Slope |  |  | Roof Slope |  |  | Roof Slope |  |  |
|  |  |  | 4:12 | 8:12 | 12:12 | 4:12 | 8:12 | 12:12 | 4:12 | 8:12 | 12:12 |
| 12" | $11 / 2 \times 71 / 4$ | Span | $14^{\prime \prime}-11^{\prime \prime}$ | $12^{\prime}-11^{\prime \prime}$ | 10' $10^{\prime \prime}$ | 13'-10" | 12'0" | $10^{\prime}-1{ }^{\prime \prime}$ | 13'0 ${ }^{\prime \prime}$ | 11'-4" | 9' $6^{\prime \prime}$ |
|  |  | Nail Oty. | 9 | 4 | 3 | 10 | 5 | 3 | 12 | 5 | 3 |
|  | $11 / 2 \times 91 / 2$ | Span | 19'-7" | $16^{\prime}-11^{\prime \prime}$ | 14'2" | 18'-2' | 15' 9" | 13'-3" | 17' 1' | 14'-10" | 12'-6" |
|  |  | Nail Oty. | 12 | 6 | 3 | 14 | 6 | 4 | 15 | 7 | 4 |
|  | 11/2 $\times 111 / 8$ | Span | 24'-5" | 21' ${ }^{\prime \prime}$ | 17'-8" | 22'-9" | 19'-9" | $16^{\prime \prime} 6^{\prime \prime}$ | 21'-5" | 18' $7^{\prime \prime}$ | 15' ${ }^{\text {" }}$ |
|  |  | Nail Oty. | 15 | 7 | 4 | * | 8 | 5 | * | 9 | 5 |
| $16 "$ | 11/2x71/4 | Span | 13'-7" | 11' ${ }^{\text {9" }}$ | 9'-10" | 12'-7" | 10'- $11^{\prime \prime}$ | 9'- ${ }^{\prime \prime}$ | 11'-10" | 10'-4" | 8' 8" |
|  |  | Nail Oty. | 11 | 5 | 3 | 13 | 6 | 4 | 14 | 7 | 4 |
|  | $11 / 2 \times 91 / 2$ | Span | 17'-9" | 15' 5" | 12'-10" | $16^{\prime \prime} 6^{\prime \prime}$ | 14'-4" | 12'-0" | $15^{\prime \prime} 6^{\prime \prime}$ | 13'-6" | $11^{\prime}-4^{\prime \prime}$ |
|  |  | Nail Oty. | 14 | 7 |  | * | 8 | 5 | * | 8 | 5 |
|  | 11/2×11/8 | Span | 22'3" | 19'3" | $16^{\prime} 1^{1 \prime}$ | 20'8' ${ }^{\prime \prime}$ | 17'-11" | 15'0 ${ }^{\text {" }}$ | 19'- ${ }^{\prime \prime}$ | 16'-11" | $14^{\prime}-2^{\prime \prime}$ |
|  |  | Nail Oty. | * | 8 | 5 | * | 9 | 6 | * | 10 | 6 |

Where number of nails is designated as "*" or resulted to more than 15 , connection shall be evaluated by a design professional.
Notes:

1. Tables are based on:

Minimum rafter bearing length of $31 / 2^{\prime \prime}$, assuming a top plate FcT of 425 psi .
Uniform load.
Simple Span.
2. Spans shown are the maximum horizontal distance from the outside face of the exterior wall to center of ridge.
3. Purlins may be installed (per section R802.5.1 of the IRC) to reduce rafter spans.
4. Interpolation to determine nail quantity for other slopes is permitted.
5. Spans developed using apparent $E$.
6. Design conditions outside the scope of this guide may be designed using CSD software.

How to Use This Table

1. Determine the roof snow load
2. Determine the rafter on-center spacing.
3. Scan down the appropriate roof snow load column until reaching a value that meets or exceeds the span of the application.
4. Select the PWLVL rafter depth and note the number of $0.131^{\prime \prime} \times 31 / 4$ " nails required at the heel and ceiling joist lap connection for the roof slope.
5. Spans developed using apparent E .

## Power Bean ${ }^{\circ}$ DESIGN PROPERTIES 3000F

| Allowable Design Stresses (psi) |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Flexural <br> Stress** <br> Fb | Tension <br> Parallel to Grain <br> Ft | Compression P <br> perpendicular to <br> Grain Fc 1 | Horizontal Shear <br> Fv | Modulus of <br> Elasticity <br> E |  |
| $3-1 / 2^{\prime \prime} \& 5-1 / 2^{\prime \prime}$ | 3000 | 1350 | 805 | 300 | $2,100,000$ |  |
| $7 "$ | 3000 | 1300 | 805 | 300 | $2,100,000$ |  |

Power Beam® Section Properties and Allowable Capacities
3-1/2" BEAM WIDIH

| Depth (in) | $9-1 / 2$ | $11-7 / 8$ | 14 | 16 | 18 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Weight* $^{\star}(\mathrm{lbs} / \mathrm{ft})$ | 9.2 | 11.6 | 13.6 | 15.6 | 17.5 |
| $\mathrm{C}_{\mathrm{db}}$ Factor (L=21') | 1.0 | 1.0 | 1.0 | 1.00 | 0.999 |
| I (in ${ }^{4}$ ) | 250 | 489 | 800 | 1195 | 1701 |
| Moment Capacity (lbs-ft) | 13161 | 20582 | 28583 | 37333 | 47193 |
| Shear Capacity (lbs) | 6650 | 8316 | 9800 | 11200 | 12600 |


| 5-1/2" BEAM WIDIH |  |
| :---: | :---: |


| Depth (in) | $9-1 / 2$ | $11-7 / 8$ | 14 | 16 | 18 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Weight^ (lbs/ft) $^{2}$ | 14.5 | 18.2 | 21.4 | 24.4 | 27.5 |
| $\mathrm{C}_{\mathrm{db}}$ Factor (L=21') | 1.00 | 0.989 | 0.989 | 0.982 | 0.976 |
| I (in ${ }^{4}$ ) | 393 | 768 | 1258 | 1877 | 2673 |
| Moment Capacity (lbs-ft) | 20682 | 32246 | 44415 | 57625 | 72503 |
| Shear Capacity (lbs) | 10450 | 13068 | 15400 | 17600 | 19800 |

7" BEAM WIDTH

| Depth (in) | $9-1 / 2$ | $11-7 / 8$ | 14 | 16 | 18 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Weight* (lbs/ft) | 18.5 | 23.1 | 27.2 | 31.1 | 35.0 |
| $\mathrm{C}_{\mathrm{db}}$ Factor (L=21') | 0.996 | 0.985 | 0.977 | 0.970 | 0.965 |
| I (in ${ }^{4}$ ) | 500 | 978 | 1601 | 2389 | 3402 |
| Moment Capacity (lbs-ft) | 24472 | 37845 | 52127 | 67631 | 85093 |
| Shear Capacity (lbs) | 13300 | 16632 | 19600 | 22400 | 25200 |
| Stock Depths | $9-1 / 2^{\prime \prime}$ | $11-7 / 8^{\prime \prime}$ | $14 "$ | $16^{\prime \prime}$ | $18{ }^{\prime \prime}$ |

* Beam Weights are based on 40 pcf.
${ }^{* *}$ Flexural Stress, Fb, shall be modified by Volume Factor, Cv, as outlined in ICC ESR-1940, APA Product report-L263 and APA-EWS Y117
where;
$\mathrm{Cv}=\mathrm{KL}\left[(21 / \mathrm{L})^{0.05} \times(12 / \mathrm{d})^{0.05} \times(5.125 / \mathrm{b})^{0.05}\right]<1.0$
where:
$K L=$ loading coefficient ( 1.0 for uniformly distributed),
$\mathrm{L}=$ length of bending member between points of zero moment, ft.,
$\mathrm{d}=$ depth of bending member, in.
$b=$ width of bending member, in.
Tabulated Moment Capacities are based on a span of 21 feet and modified for other spans.
Width and depth portions of Volume Factor, Cv, are incorporated in tabulated Moment Capacities using Cab Factor.
Note: Allowable design properties and load capacities are based on a load duration of 100 percent and dry use conditions.


# Рошег Beam ${ }^{\circ}$ <br> DESIGN PROPERTIES 3000F 

## VERTICAL HOLES

Whenever possible, avoid drilling vertical holes through glulam beams. As a rule of thumb, vertical holes drilled through the depth of a glulam beam will cause a reduction in the capacity at the location directly proportional to the ratio of $1-1 / 2$ times the diameter of the hole to the width of the beam. For example a one inch drilled hole in a 6 -inch wide beam would reduce the capacity of the beam at that section by approximately $\frac{(1 \times 1-1 / 2)}{6}=25 \%$
For this reason, when it is necessary to drill vertical holes through a glulam member, the holes should be positioned in areas of the member that are stressed to less than 50 percent of design in bending. In a simply supported, uniformally loaded beam, this area would be located from the end of the beam inward approximately $1 / 8$ of the beam span. In all cases, the minimum clear edge distance, as measured from either side of the member to the nearest edge of the vertical hole, should be 2-1/2 times the hole diameter. Use a drill guide to minimize " wandering" of the bit as it passes through knots or material of varying density, and to insure a true alignment of the hole through the depth of the beam.

## HORIZONTAL HOLES

Like notches, holes in a glulam beam remove wood fiber, thus reducing the net area of the beam at the hole location and introducing stress concentrations. These effects cause a reduction in the capacity of the beam in the area of the penetration. For this reason, horizontal holes in glued laminated timbers are limited in size and location to maintain the structural integrity of the beam. Figure 1 shows the zones of a uniformly loaded, simply supported beam where the field drilling of holes may be considered. These non-critical zones are located in portions of the beam stressed to less than 50 percent of design bending stress and less than 50 percent of design shear stress. For beams of more complex loading or other than simple spans, similar diagrams may be developed.

ZONES WHERE SMALL HORIZONTAL HOLES ARE PERMITTED IN A UNIFORMLY LOADED, SIMPLY SUPPORTED BEAM


Zones where horizontal holes are permitted for passage of wires, conduit, etc.

$$
\ell=\text { length of beam } \quad d=\text { length of beam }
$$

Field-drilled holes should be used for access only and should not be used as attachment points for brackets or other load bearing hardware unless specifically designed as such by the engineer or designer. Examples of access holes include those used for the passage of wires, electrical conduit, small diameter sprinkler pipes, fiber optic cables, and other small, lightweight materials. These field-drilled horizontal holes should meet the following guidelines:

1. Hole size: the hole diameter should not exceed 1-1/2 inches or $1 / 10$ the beam depth, whichever is smallest, with the exception of 1 -inch-diameter or smaller holes as noted in Item 2 below.
2. Hole location: The hole should have a minimum clear distance, as measured from the edge of the hole to the nearest of the beam, of 4 hole diameters to the top or bottom face of the beam and 8 hole diameters from the end of the beam. Note that the horizontal hole should not be drilled in the moment-critical zone, as defined in the figure above, unless approved by an engineer or architect qualified in engineered timber design.

## Poшer Beam ALLOWABLE FLOOR LOAD TABLES LDF=1.0-3000F

These tables can be used to size simple span beams and headers that carry uniform loads. The PLF loads must be calculated and take into account all floor and roof framing loads coming onto the beam or header.
Key: For each clear span there are three numbers:

| ALLOWABLE FLOOR LOAD TABLES LDF=1.0-3000F |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{array}{\|l\|} \hline \text { Actual } \\ \text { Span } \end{array}$ |  |  |  |  |  | 5-1/2" |  |  |  |  |
|  |  |  | th (in.) |  |  |  |  | (in.) |  |  |
|  | 9-1/2 | 11-718 | 14 | 16 | 18 | 9-1/2 | 11-7/8 | 14 | 16 | 18 |
| 7 | 2149 | 3311 | 4200 | 5169 | 6300 | 3377 | 5203 | 6600 | 8123 | 9900 |
|  | 2149 3 | 3311 4.5 | 4200 6 | 5169 7.5 | 6300 9 | 3377 3 | 5203 4.5 | 6600 6 | 8123 7.5 | 9900 9 |
|  | 1645 | 2571 | 3459 | $\stackrel{7200}{ }$ | 5040 | 2585 | 4040 | 5435 | 6600 | 7920 |
| ${ }^{8}$ | 1520 | 2571 | 3459 | 4200 | 5040 | 2388 | 4040 | 5435 | 6600 | 7920 |
|  | ${ }_{3}$ | 4.5 | , | 7.5 | ${ }^{9}$ | 8 | 4.5 | 6 | 7.5 | ${ }_{9}$ |
| ${ }^{9}$ | 1300 | 2031 | 2823 | 3537 | 4200 | 2043 | 3192 | 4436 | 5558 | 6600 |
|  | 1067 | 2031 | 2833 | 3537 | 4200 | 1677 | 3192 | 4436 | 5558 | 6600 |
|  | 3 | 4.5 | 6 | 6 | 7.5 | 3 | 4.5 | 6 | 6 | 7.5 |
| 10' | 1053 | 1645 | 2287 | 2987 | 3600 | 1655 | 2585 | 3593 | 4693 | 5657 |
|  | 778 | 1520 | 2287 | 2987 | 3600 | 1223 | 2388 | 3593 | 4693 | 5657 |
|  | 3 | 3 | 4.5 | 6 | 7.5 | 3 | 3 | 4.5 | 6 | 7.5 |
| ${ }^{11}$ | 870 | 1360 | 1890 | 2468 | 3124 | 1367 | 2137 | 2970 | 3879 | 4909 |
|  | 585 3 | 1142 3 | 1871 4.5 | 2468 6 | 3124 <br> 7.5 | 919 3 | 1794 3 | 2940 4.5 | 3879 6 | 4909 7.5 |
| ${ }^{12}$ | 675 | 1142 | 1588 | 2074 | 2625 | 1061 | 1795 | 2495 | 3259 | 4125 |
|  | 450 | 879 | 1441 | 2074 | 2625 | 707 | 1382 | 2264 | 3259 | 4125 |
|  | 1.5 | 3 | 4.5 | 6 | 6 | 1.5 | 3 | 4.5 | 6 | 6 |
| ${ }^{13}$ | 531 | ${ }^{973}$ | ${ }^{1353}$ | 1767 | 2237 | 835 | 1530 | 2126 | 2777 | 3515 |
|  | 354 | 692 | 1133 | 1692 | 2237 | 556 | 1087 | 1781 | 2658 | 3515 |
|  | 1.5 | 3 | 4.5 | 4.5 | 6 | 1.5 | 3 | 4.5 | 4.5 | 6 |
| 14' | 425 | 831 | 1167 | 1524 | 1929 | 668 | 1305 | 1833 | 2395 | 3020 |
|  | 284 1.5 | ${ }_{5}^{54}$ | ${ }_{9}^{907}$ | 1354 4.5 | 1929 6 | 446 1.5 | 870 3 | 1426 | 2128 | 3020 |
| ${ }^{15}$ | ${ }^{346}$ | 675 | 1016 | 13.8 | 1680 | $\stackrel{1.5}{543}$ | 1061 | 1597 | 2084 | 2622 |
|  | 231 | 450 | 738 | 1101 | 1568 | 362 | 707 | 1159 | 1731 | 2464 |
|  | 1.5 | 3 | 3 | 4.5 | 6 | 1.5 | 3 | 3 | 4.5 | 6 |
| 16' | 285 | 556 | 893 | 1167 | 1477 | 448 | 874 | 1404 | 1825 | 2297 |
|  | 190 | 371 | 608 | 907 | 1292 | 298 | 583 | 955 | 1426 | 2030 |
|  | 1.5 |  | 3 | 4.5 | 4.5 | 1.5 |  |  | 45 | 4.5 |
| ${ }^{17}$ | $\stackrel{238}{238}$ | 464 | 760 507 | $\begin{array}{r}1033 \\ \\ \\ \hline 57\end{array}$ | ${ }^{1308}$ | 373 | 729 | ${ }^{1195}$ | 1612 | $\begin{array}{r}2028 \\ 163 \\ \hline 1\end{array}$ |
|  | 158 | 309 | 507 | 757 | 1077 | 249 | 486 | 796 | 1189 | 1693 |
|  | 1.5 | 1.5 | 3 | 4.5 | 4.5 | 1.5 | 1.5 | 3 | 4.5 | 4.5 |
| ${ }^{18}$ | 200 133 | 391 | 640 | 922 | 1167 | 314 | 614 | 1006 | 1434 | 1804 |
|  | 133 | 261 | 427 | 637 | 907 | 210 | 409 | 671 | 1001 | 1426 |
|  | 1.5 | 1.5 | 3 | 3 | 4.5 | 1.5 | 1.5 | 3 | 3 | 4.5 |
| ${ }^{19}$ | 170 | 332 | 545 | 813 | 1047 | 267 | 522 | 856 | 1277 | 1615 |
|  | 113 | 222 | 363 | 542 | 772 | 178 | 348 | 570 | 852 | 1212 |
| $20^{\prime}$ | 1.5 146 | 1.5 285 | $\stackrel{3}{467}$ | 3 697 | 4.5 | 1.5 229 | 1.5 | 73 | + | $\begin{array}{r}14.5 \\ 1454 \\ \hline\end{array}$ |
|  | 97 | 190 |  |  |  |  |  |  |  |  |
|  | 1.5 | 1.5 | 3 | 4 | 4.5 | 193 1.5 | 1.5 <br> 1.5 | 489 3 | 730 3 | $\begin{array}{r}1040 \\ 4.5 \\ \hline\end{array}$ |
| $21^{1}$ | 126 | 246 | ${ }^{403}$ | 602 | 856 | 198 | 387 | 634 | 946 | 1315 |
|  | 84 | 164 | 269 | 401 | 571 | 132 | 258 | 422 | 631 | 898 |
|  | 1.5 | 1.5 | 3 | 3 | 4.5 | 1.5 | 1.5 | 3 | 3 | 4.5 |
| ${ }^{22}$ | 110 | 214 | 351 | 524 | 745 | 172 | 336 | 551 | ${ }^{823}$ | 1171 |
|  | 73 | 143 | 234 | 349 | 497 | 115 | 224 | 367 | 549 | 781 |
| ${ }^{23}$ | 1.5 | 1.5 | 1.5 307 | 458 | ${ }^{6}$ | 1.5 | 1.5 | 4.5 | 32 |  |
|  | 64 | 125 | 205 | 305 | 435 | 100 | 196 | 422 | 480 | ${ }_{683}^{125}$ |
|  | 1.5 | 1.5 | 1.5 | 3 | 3 | 1.5 | 1.5 | 1.5 | 3 | 3 |
| 24' | 84 | 165 | 270 | ${ }^{403}$ | 574 | 138 | ${ }^{259}$ | ${ }^{425}$ | 634 | 902 |
|  | 56 | 110 | 180 | 269 | 383 | 88 | 173 | 283 | 422 | 602 |
|  | 1.5 | 1.5 | 1.5 | 3 | 3 | 1.5 | 1.5 | 1.5 | 3 | 3 |
| ${ }^{25}$ | 75 | 146 | 239 | 357 | 508 | 117 | 229 | 376 | 561 | 798 |
|  | 50 1.5 | 97 1.5 | 159 1.5 | ${ }^{238}$ | ${ }^{339}$ | 78 | 153 | 250 | 374 | 532 |
| ${ }^{26}$ | 59 | 116 | 190 | 283 | 403 | ${ }^{73}$ | 182 | 1.5 298 | 445 | 634 |
|  | 40 | 77 | 127 | 189 | 269 | 62 | 121 | 199 | 297 | 422 |
|  | 1.5 | 1.5 | 1.5 | 1.5 | 3 | 1.5 | 1.5 | 1.5 | 1.5 | 3 |
| ${ }^{27}$ | 66 | 130 | 212 | 317 | 452 | 104 | 204 | 334 | 498 | 710 |
|  | 44 | 86 | 142 | 211 | 301 | 70 | 136 | 223 | 332 | 473 |
|  | 1.5 |  | 1.5 | 1.5 |  | 1.5 | 1.5 | 1.5 | 1.5 |  |
| ${ }^{28}$ | 53 | 104 | 170 | 254 | ${ }^{362}$ | 84 | 163 | 267 | 399 | ${ }_{5}^{568}$ |
|  | 35 1.5 | 1.59 1.5 | 113 1.5 | 169 1.5 | $\begin{array}{r}241 \\ 3 \\ \hline\end{array}$ | 56 1.5 | 109 1.5 | 178 1.5 | 266 1.5 | 379 3 |
| $29^{\prime}$ |  | ${ }^{93}$ | 153 | 229 | ${ }^{325}$ | 75 | 147 | ${ }^{241}$ | 359 | 511 |
|  | 32 1.5 | 62 1.5 | 102 1.5 | 152 1.5 | ${ }_{2}^{217}$ | 50 1.5 | 98 1.5 1.5 | 160 1.5 | 239 1.5 | 341 3 |
| $30^{\prime}$ | 43 | 84 | 138 | 206 | 294 | 68 | 133 | 217 | 324 | 462 |
|  | 29 15 | 56 15 | 15 | 138 | 196 | 45 | 88 | 145 | 215 | 308 |
| $32^{\prime}$ | 36 | 70 | 1.14 | 170 | 242 | ${ }^{\text {1.5 }}$ | 1.5 109 | 1.5 179 | 1.5 267 | 381 |
|  | 24 | 45 | 76 | 113 | 161 | 37 | 73 | 119 | 178 | ${ }_{254}$ |
|  | 1.5 | 1.5 | 1.5 | 1.5 | 1.5 | 1.5 | 1.5 | 1.5 | 1.5 | 1.5 |

## Power Column ${ }^{\circledR}$ COMBINATION \#50

## FEATURES:

- Combination \#50 (\#1 Dense SYP)
- $\mathrm{MOE}=1.9 \times 10 \mathrm{psi}$
- $\mathrm{Fb}=2100-2300 \mathrm{psi}$
- $\mathrm{Fc}=1700-2300 \mathrm{psi}$
- Treated Columns Available


## Stocked at Coastal Forest Products Up to 48' Lengths

| $3-1 / 2^{\prime \prime} \times 3-1 / 2^{\prime \prime}$ | $5-1 / 2^{\prime \prime} \times 5-1 / 2^{\prime \prime}$ |
| :--- | :--- |
| $3-1 / 2^{\prime \prime} \times 5-1 / 2^{\prime \prime}$ | $5-1 / 2^{\prime \prime} \times 7$ " |
| $3-1 / 2^{\prime \prime} \times 7$ " | $7^{\prime \prime} \times 7$ " |

$3-1 / 2$ " $\times 7$ "
$7 " \times 7$ "


## Power Column ${ }^{\circledR}$ COMBINATION \#50

## Allowable Axial Loads (Pounds) for Combination No. 50

Side loads are not permitted. End loads are limited to a maximum eccentricity of either $1 / 6$ column width or depth, whichever is worse.


| Effective Column Length (ft) | Lamination Net Width = 5-1/2" |  |  |  |  |  | Lamination Net Width = 7" <br> Net Depth = 7" (6 lams) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Net Depth = 5-1/2" (4 lamas) |  |  | Net Depth = 7" (6 lams) |  |  |  |  |  |
|  | Load Duration Factor |  |  | Load Duration Factor |  |  | Load Duration Factor |  |  |
|  | 1.00 | 1.15 | 1.25 | 1.00 | 1.15 | 1.25 | 1.00 | 1.15 | 1.25 |
| 6 | 32,920 | 36,550 | 38,810 | 45,610 | 51,260 | 54,840 |  |  |  |
| 8 | 27,420 | 29,640 | 30,950 | 39,290 | 42,590 | 44,520 | 53,480 | 59,380 | 63,060 |
| 10 | 21,970 | 23,280 | 24,000 | 31,680 | 33,560 | 34,650 | 46,900 | 51,070 | 53,550 |
| 12 | 17,550 | 18,380 | 18,850 | 25,300 | 26,470 | 27,140 | 40,070 | 42,840 | 44,450 |
| 14 | 14,200 | 14,760 | 15,080 | 20,430 | 21,210 | 21,660 | 38,840 | 35,730 | 36,830 |
| 16 | 11,670 | 12,060 | 12,290 | 16,760 | 17,300 | 17,610 | 28,630 | 29,990 | 30,770 |
| 18 | 9,730 | 10,020 | 10,180 | 13,950 | 14,350 | 14,580 | 24,400 | 25,400 | 25,980 |
| 20 | 8,230 | 8,440 | 8,570 | 11,780 | 12,080 | 12,250 | 20,980 | 21,740 | 22,180 |
| 22 | 7,040 | 7,210 | 7,300 | 10,070 | 10,290 | 10,420 | 18,190 | 18,780 | 19,120 |
| 24 | ---- | ---- | ---- | ----- | ----- | ----- | 15,900 | 16,370 | 16,640 |

## NOTES and Allowable Design Properties

1. The tabulated allowable loads apply to one-piece glulam members made with all N1D14 laminations (Combination 50 ) without special tension laminations.
2. Applicable service conditions = dry.
3. The tabulated allowable loads are based on simply axially loaded columns subjected to a maximum eccentricity of either $1 / 6$ column width or $1 / 6$ column depth, whichever is worse. For side loads, other eccentric end loads, or other combined axial and flexural loads, see 2005 NDS.
4. The column is assumed to be unbraced, except at the column ends, and the effective column length is equal to the actual column length.
5. Design properties for normal load duration and dry-use service conditions:

- Compression parallel to grain $\left(\mathrm{Fc}_{\mathrm{c}}\right)=2,300$ psi for 4 or more lams, or $1,700 \mathrm{psi}$ for 2 or 3 lams.
- Modulus of elasticity $(E)=1.9 \times 10 \mathrm{psi}$.
- Flexural stress when loaded parallel to wide faces of lamination (Fby)
$=2,300$ psi for 4 or more lams, or 2,100 psi for 3 lams.
- Flexural stress when loaded perpendicular to wide faces of lamination (Fbx) $=2,100$ psi for 2 lams to $15^{\prime \prime}$ deep without special tension laminations.
- Volume factor for $\mathrm{F}_{\mathrm{bx}}$ is in accordance with 2005 NDS. Size factor for $\mathrm{F}_{\text {by }}$ is $(12 / \mathrm{d})^{1 / 2}$, where $d$ is equal to the lamination width inches.


## POWER PRESERVED GLULAM ${ }^{\ominus}$ CLEAR GUARD ${ }^{\text {TM }}$ TREATED GLULAMS

## POWER PRESERVED GLULAM ${ }^{\circledR}$ (PPG)

Anthony Forest Products ${ }^{\otimes}$ has been a name to trust in the glued laminated timber business for over 45 years. Anthony stock $2400 \mathrm{Fb}-2.1 \mathrm{E}-300 \mathrm{Fv}$ SYP glulam has been our mainstay in business along with the high strength Power Beam ${ }^{\circledR} 3000 \mathrm{Fb}$ $-1.8 \mathrm{E}-300 \mathrm{Fv}$ IJC beam.

With the shortage of high-quality, high strength, solid southern pine treated timber, Anthony offers Power Preserved Glulam ${ }^{\circledR}$ Beams, which have been pressure treated with Hoover Cop-Guard ${ }^{\circledR}$ or Clear-Guard ${ }^{\text {TM }}$ at .04 pounds per cubic foot (PCF) or .02 pounds per cubic foot retention levels suitable for above ground uses respectively. Power Preserved Glulam ${ }^{\oplus}$ products will resist fungal decay and wood-destroying insect attacks and are covered by a 25 year warranty by Hoover.

Cop-Guard ${ }^{\oplus}$ (Copper Naphthenate-CuN and Clear-Guard ${ }^{\text {m }}$ (PBC/Permethrin) wood preservatives are both dissolved in low odor mineral spirits as a carrier and are an ideal fungicide and insecticide for the long term preservation of wood products. PPG beams and columns have a green coloration when treated with Cop-Guard ${ }^{\text {® }}$ and have no real color change when treated with Clear-Guard ${ }^{\text {TM }}$ wood preservatives.

Clear-Guard ${ }^{\text {TM }}$ wood preservative treated glulam is in a solution of IPBC (fungicide) and Permethrin (insecticide) wood preservative listed in AWPA P-58-10. Both preservatives are low in toxicity, environmentally safe, and non-corrosive to fasteners.

- Three times as strong as \#2 PT SYP $4 \times 12$
- No strength reductions required after treatment.
- Automatic substitute for Parallam ${ }^{\ominus}$ Plus PSL.
- Stainable and Paintable (See restrictions).
- Not considered hazardous material

| Stocked at Coastal Forest Products Up to 48' Lengths |  |
| :--- | :--- |
| $3-1 / 2^{\prime \prime} \times 9-1 / 2^{\prime \prime}$ | $5-1 / 4^{\prime \prime} \times 9-1 / 2^{\prime \prime}$ |
| $3-1 / 2^{\prime \prime} \times 11-7 / 9^{\prime \prime}$ | $5-1 / 4^{\prime \prime} \times 11-7 / 8^{\prime \prime}$ |



## CONDITIONS OF USE (DRY OR WET)

Power Preserved Glulam ${ }^{\oplus}$ products are recommended for above ground use where the equilibrium moisture content (EMC) of the laminated beam will not exceed $16 \%$ thus allowing dry-use design values (over $16 \%$ considered wet-use.) The definitions of dry and wet service vary from the many publications available on the subject.

## CODE APPROVALS

Power Preserved Glulam ${ }^{\circledR}$ is manufactured in accordance with ANSIA190.1, which is the code recognized standard for glued laminated timber and is accepted nationwide under the CC-ESR 1940 and APA Product Report L282. The adhesive used in our glulam conforms to wet-use complying with ASTM D2559. The APA-EWS is our third party inspection agency.

## FACT SHEET

- $2400 \mathrm{Fb}-1.8 \mathrm{E}-300 \mathrm{Fv}$ SYP glulam industrial grade.
- High strength allows for reduction in size columns or number of pilings and piers.
- Two separate warranties for your protection.
- Balanced lay-up and zero camber.
- No top or bottom.
- As environmentally safe as untreated wood.
- Above ground use for beams (AWPA use categories UC3B) and ground contact for the columns (AWPA use categories UC4A, UC4B and UC4C).
- For PPG Beams sizes not listed, please call Anthony Forest.


## FASTENERS

- Non-Corrosive fasteners may be used with PPG in protected areas.
- Corrosion resistant fasteners are required if a connection is made to other water borne copper treated wood.
- Local building code requirements will always supersede above restrictions.
- Above ground use for beams (AWPA use categories UC3B) and ground contact for the columns (AWPA use categories UC4A, UC4B and UC4C).
- For PPG Beams sizes not listed, please call Anthony Forest.



# POWER PRESERVED GLULAM ${ }^{\circledR}$ <br> CLEAR GUARD ${ }^{\text {TM }}$ TREATED GLULAMS 

## Treated Glulam Allowable Floor Loads (plf)

EWS 24F-V5M1/SP • Dry-Use $\cdot F_{b}=2,400 \mathrm{psi} \cdot \mathrm{F}_{\mathrm{v}}=300 \mathrm{psi} \cdot E=1.8 \times 10^{6} \mathrm{psi} \cdot \mathrm{F}_{\mathrm{c}}=740 \mathrm{psi} \cdot(\mathrm{LDF}=1.00)$

| Width <br> (in) | Depth <br> (in) | Load Condition | Span (feet) |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | 6 | 8 | 10 | 12 | 14 | 16 | 18 | 20 | 22 | 24 | 26 |
| 3-1/2" | $91 / 2$ | Total Load | 2108 | 1293 | 827 | 474 | 298 | 200 | 140 | 102 | 77 | 59 | 47 |
|  |  | Live Load | --- | 1279 | 655 | 379 | 239 | 160 | 112 | 82 | 62 | 47 | 37 |
|  |  | Min. End/Int.Bearing (in.) | 2.5/6.3 | 2.0/5.0 | 1.6/4.0 | 1.5/3.8 | 1.5/3.8 | 1.5/3.8 | 1.5/3.8 | 1.5/3.8 | 1.5/3.8 | 1.5/3.8 | 1.5/3.8 |
|  | $117 / 8$ | Total Load <br> Live Load <br> Min. End/Int.Bearing (in.) | $\begin{gathered} 2901 \\ --- \\ 3.4 / 8.5 \end{gathered}$ | 1918 <br> 3.0/7.5 | $\begin{gathered} 1293 \\ 1279 \\ 2.5 / 6.3 \\ \hline \end{gathered}$ | $\begin{gathered} 898 \\ 740 \\ 2.1 / 5.3 \end{gathered}$ | $\begin{gathered} 583 \\ 466 \\ 1.6 / 4.0 \end{gathered}$ | $\begin{gathered} 390 \\ 312 \\ 1.5 / 3.8 \end{gathered}$ | $\begin{gathered} 274 \\ 219 \\ 1.5 / 3.8 \end{gathered}$ | $\begin{gathered} 200 \\ 160 \\ 1.5 / 3.8 \end{gathered}$ | $\begin{gathered} 150 \\ 120 \\ 1.5 / 3.8 \end{gathered}$ | $\begin{gathered} 116 \\ 93 \\ 1.5 / 3.8 \end{gathered}$ | $\begin{gathered} 91 \\ 73 \\ 1.5 / 3.8 \end{gathered}$ |
|  | 14 | Total Load | 3743 | 2401 | 1782 | 1248 | 917 | 702 | 449 | 328 | 246 | 190 | 149 |
|  |  | Live Load |  |  | 1784 | 1213 | 764 | 512 | 359 | 262 | 197 | 152 | 119 |
|  |  | Min. End/Int.Bearing (in.) | 4.4/11.0 | 3.8/9.5 | 3.5/8.8 | 2.9/7.3 | 2.8/7.0 | 2.2/5.5 | 1.6/4.0 | 1.5/3.8 | 1.5/3.8 | 1.5/3.8 | 1.5/3.8 |
|  | 16 | Total Load Live Load Min. End/Int.Bearing (in.) | $\begin{gathered} 4719 \\ ---14.0 \end{gathered}$ | $\begin{gathered} 2926 \\ ---11.5 \end{gathered}$ | $\begin{gathered} 2101 \\ --1 \\ 4.1 / 10.3 \end{gathered}$ | 1615 <br> 3.8/9.5 | $\begin{gathered} 1182 \\ 1140 \\ 3.3 / 8.3 \end{gathered}$ | $\begin{gathered} 901 \\ 764 \\ 2.8 / 7.0 \end{gathered}$ | $\begin{gathered} 671 \\ 537 \\ 2.4 / 6.0 \end{gathered}$ | $\begin{gathered} 489 \\ 391 \\ 1.9 / 4.8 \end{gathered}$ | $\begin{gathered} 367 \\ 294 \\ 1.6 / 4.0 \end{gathered}$ | $\begin{gathered} 283 \\ 226 \\ 1.5 / 3.8 \end{gathered}$ | $\begin{gathered} 223 \\ 178 \\ 1.5 / 3.8 \end{gathered}$ |
|  | 18 | Total Load | 5917 | 3522 | 2485 | 2046 | 1499 | 1143 | 899 | 725 | 523 | 403 | 317 |
|  |  | Live Load | --- | --- | --- | --- | --- | 1088 | 764 | 557 | 418 | 322 | 253 |
|  |  | Min. End/Int.Bearing (in.) | 7.0/17.5 | 5.5/13.8 | 4.9/2.3 | 4.8/12.0 | 4.1/10.3 | 3.6/9.0 | 3.2/8.0 | 2.8/7.0 | 2.3/5.8 | 1.9/4.8 | 1.6/4.0 |
| 5-1/4" | $91 / 2$ | Total Load <br> Live Load <br> Min. End/Int.Bearing (in.) | $\begin{gathered} 3199 \\ --- \\ 2.5 / 6.3 \end{gathered}$ | $\begin{gathered} 1948 \\ 1181 \\ 2.0 / 5.0 \end{gathered}$ | $\begin{gathered} 1264 \\ 605 \\ 1.6 / 4.0 \end{gathered}$ | $\begin{gathered} 719 \\ 350 \\ 1.5 / 3.8 \end{gathered}$ | $\begin{gathered} 453 \\ 220 \\ 1.5 / 3.8 \end{gathered}$ | $\begin{gathered} 303 \\ 148 \\ 1.5 / 3.8 \end{gathered}$ | $\begin{gathered} 214 \\ 104 \\ 1.5 / 3.8 \end{gathered}$ | $\begin{gathered} 156 \\ 76 \\ 1.5 / 3.8 \end{gathered}$ | $\begin{gathered} 117 \\ 57 \\ 1.5 / 3.8 \end{gathered}$ | $\begin{gathered} 90 \\ 44 \\ 1.5 / 3.8 \end{gathered}$ | $\begin{gathered} 71 \\ 34 \\ 1.5 / 3.8 \end{gathered}$ |
|  | $117 / 8$ | Total Load | 4403 | 2910 | 1944 | 1344 | 885 | 593 | 419 | 305 | 229 | 177 | 139 |
|  |  | Live Load | --- | --- | --- | 1131 | 712 | 477 | 335 | 244 | 183 | 141 | 111 |
|  |  | Min. End/Int.Bearing (in.) | 3.4/8.5 | 3.0/7.5 | 2.5/6.3 | 2.1/5.3 | 1.6/4.0 | 1.5/3.8 | 1.5/3.8 | 1.5/3.8 | 1.5/3.8 | 1.5/3.8 | 1.5/3.8 |
|  | 14 | Total Load <br> Live Load <br> Min. End/Int.Bearing (in.) | $\begin{gathered} 5679 \\ ---1.0 \end{gathered}$ | 3644 <br> 3.8/9.5 | $\begin{gathered} 2707 \\ --- \\ 3.5 / 8.8 \end{gathered}$ | $\begin{gathered} 1874 \\ 1853 \\ 2.8 / 7.0 \end{gathered}$ | $\begin{gathered} 1371 \\ 1167 \\ 2.8 / 6.3 \end{gathered}$ | $\begin{gathered} 1044 \\ 782 \\ 2.2 / 5.5 \end{gathered}$ | $\begin{gathered} 682 \\ 549 \\ 1.6 / 4.0 \end{gathered}$ | $\begin{gathered} 497 \\ 400 \\ 1.5 / 3.8 \end{gathered}$ | $\begin{gathered} 373 \\ 301 \\ 1.5 / 3.8 \end{gathered}$ | $\begin{gathered} 289 \\ 232 \\ 1.5 / 3.8 \end{gathered}$ | $\begin{gathered} 228 \\ 182 \\ 1.5 / 3.8 \end{gathered}$ |
|  | 16 | Total Load | 7161 | 4440 | 3188 | 2425 | 1794 | 1400 | 1018 | 742 | 558 | 460 | 340 |
|  |  | Live Load | --- | --- | --- | --- | 1741 | 1167 | 819 | 597 | 449 | 346 | 272 |
|  |  | Min. End/Int.Bearing (in.) | 5.6/14.0 | 4.6/11.5 | 4.1/10.3 | 3.8/9.5 | 3.3/8.3 | 2.8/7.0 | 2.4/6.0 | 1.9/4.8 | 1.6/4.0 | 1.5/3.8 | 1.5/3.8 |
|  | 18 | Total Load <br> Live Load <br> Min. End/Int.Bearing (in.) | $\begin{gathered} 8979 \\ ---9 / 17.5 \end{gathered}$ | $\begin{gathered} 5343 \\ -- \\ 5.5 / 13.8 \end{gathered}$ | $\begin{gathered} 3770 \\ --- \\ 4.912 .3 \end{gathered}$ | $\begin{gathered} 3106 \\ -- \\ 4.8 / 12.0 \end{gathered}$ | $\begin{gathered} 2274 \\ -- \\ 4.1 / 10.3 \end{gathered}$ | $\begin{gathered} 1734 \\ 1661 \\ 3.6 / 9.0 \end{gathered}$ | $\begin{gathered} 1365 \\ 1167 \\ 3.2 / 8.0 \end{gathered}$ | $\begin{gathered} 1128 \\ 851 \\ 2.8 / 7.0 \end{gathered}$ | $\begin{gathered} 794 \\ 639 \\ 2.3 / 5.8 \end{gathered}$ | $\begin{gathered} 615 \\ 492 \\ 1.9 / 4.8 \end{gathered}$ | $\begin{gathered} 484 \\ 387 \\ 1.6 / 4.0 \end{gathered}$ |

## NOTES:

1. Values shown are the maximum uniform loads (beam weight included) in pounds per linear foot (PLF) that can be applied to the beam.
2. These tables are for preliminary design when considering load and other conditions.

The final design should include complete design analysis.
3. Bearing lengths shown in the third row of each cell are for maximum PLF loads for the two end bearings and for the middle or intermediate bearings when beam is continuous. A shorter bearing may be used if proper analysis is done.
4. Live load is based on the deflection criterion of $L / 360$ and includes the beam weight ( 48 pcf ).
5. Total load is based on the deflection criterion with a LL/DL ration of 4 or higher.
6. For deflection limits of $L / 240$ and $L / 480$, multiply the live load figures by 1.5 and 0.75 respectfully.
7. The beam is assumed to be loaded on the top edge and with full lateral support at bearing points.
8. Selected beam must satisfy both live and total load.
9. Where no live load shows, live load is the same as total load.
10. Call Coastal Forest Products for sizes not listed.


## POWER PRESERVED COLUMN®

## PRESSURE TREATED COLUMNS

Stocked at Coastal Forest Products Up to 28' Lengths
$5-1 / 4^{\prime \prime} \times 51 / 2^{\prime \prime}$ - up to $28^{\prime}$

## POWER PRESERVED COLUMN®

Anthony Forest Products offers our popular Power Column ${ }^{\circledR}$ as a Power Preserved Column ${ }^{\circledR}$ for ground contact using Hoover Cop-Guard ${ }^{\text {® }}$. These columns are treated to the high retention level of 0.075 PCF, meeting AWPA use categories $4 \mathrm{~A}, 4 \mathrm{~B}$ and 4 C (should not be used in direct contact with water).

## Suggested Uses: (Exterior only)

- Deck support columns and boardwalks
- Residential and commercial exposed structural columns
- Raised coastal construction supports replacing piling
- Industrial and farming applications
- Pedestrian bridges and park shelters
- Pergolas


| Power Preserved Column Design Values¹ |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Combination \#50 <br> \#1 Dense SYP | $\mathrm{F}_{\mathrm{b}} \mathbf{x - x} \mathbf{a x i s}$ | $F_{b} \mathbf{y}$-y axis |  | MOE | Compression <br> Parallel to Grain $_{\mathbf{c} 1}=$ |  |
|  | $\mathrm{F}_{\mathrm{b}}$ | 3 laminations | 4 or more laminations |  | 3 laminations | 4 or more laminations |
| Design Value | 2,100 psi | 2,100 | 2,300 | $1.9 \times 10^{6}$ | 1,700 | 2,300 |
| Wet-Use Factor | 0.8 | 0.8 | 0.8 | 0.833 | 0.53 | 0.73 |
| ${ }^{1}$ The tabulated values are for moisture content of less than 16\%. Apply wet-use adjustment factors for columns in direct contact with the ground. Use of column bases or standoff may allow for dry-use. |  |  |  |  |  |  |



Coastal offers the widest offering of high quality, engineered wood products that are in-stock, protected from the environment, and shipped promptly when you need it.

## THE MOST POWERFUL SOFTWARE TOOLS IN THE MARKET

## The Most Powerful Software Tools in the Market iStruct ${ }^{\oplus}$ software suite, featuring isPlan ${ }^{\ominus}$ and isDesign ${ }^{\ominus}$

Coastal Forest Products provides customers with the best information services in the industryand supplies its customer base with software tools to perform daily engineering and drawing functions required in today's market.

## isPlan ${ }^{\circledR}$ features:

- Draw and design floor and roof framing plans with engineered wood products
- Includes structural analysis and reporting, take-offs, quotes, and cutting optimization with inventory integration
- Automatically develops loads and produces bold, color graphic layouts in 2D and 3D
- Specially engineered for companies with a dedicated design staff
- Supports the full Coastal product line
- Includes isDesign-the single member beam design


## isDesign ${ }^{\circledR}$ features:

- A user-friendly, single-member sizing program with impeccable graphics that creates easy-to-read beam calcs
- Analyze loads and calculate sizes and spacing for Coastal engineered wood products
- Requires little or no training for the architect, engineer, or designer


## Coastal customers receive:

- No charge for isDesign ${ }^{\circledR}$ single-member sizing software
- No charge for customers to distribute isDesign ${ }^{\circledR}$ to its customer base
- Customer product logos and nomenclature on beam calcs
- Printed calc sheets display shear, deflection, moment, and reaction
- Value-engineered framing plans
- Engineered or non-engineered placement plans
- Internet software training and support
- Internet updates for all software


The iStruct ${ }^{\circledR}$ software suite is truly a solution like no other and is designed for quick learning and application. The accelerated training time means users are up and running quickly and cost effectively!

## What you get from Coastal Forest Products is what your customers expect from you-the best tools and the best service possible!

## STOCK REFERENCE GUIDE

NORDIC
STRUCTURES
2"X3" Solid Flanges
Spans Up To 9-1/2" - 17' 00 " 11-7/8" $-20^{\prime} 03^{\prime \prime}$

$3 / 8$ " OSB Web
$2 " \times 3^{\prime \prime}$
$\mathrm{NI}-80$
2" X 4" Solid Flanges
Spans Up To 9-1/2"-19' $01^{\prime \prime}$ 11-7/8" - 22' 08"
14"-25' 09"
16"-28' 06"



3/8" OSB Web 2"x4"
$\mathrm{NI}-60$
2" X 3" Solid Flanges
Spans Up To
11-7/8"-20' 8"
14"-23' 06"
16" - 26' 00"

N-90
2" X 4" Solid Flanges Spans Up To 11-7/8" - 23' $03^{\prime \prime}$ 14"-26" 06" 16"-29' 03"


3/8" OSB Web 2" $\times 3$ "


3/8" OSB Web
2" $\times 4$ "
***All spans base on typical residential 40/10 loading, I/480 16" O/C


2" X 4" LVL Flanges
Spans Up To
11-7/8"-23' 11" 16" - $30^{\prime} 01^{\prime \prime}$


PWT TREATED

1-3/4" 2.1 E CP-Lam
Available Sizes (inches) 3100Fb


SIMPSON
StrongTie
Matching Connectors In-Stock!!!



## STOCK REFERENCE GUIDE

Q,TRIFOREE
24" TRIMMABLE

| Maximum Size of Pipes, Ducts and Cable Trays Through Diagonal Web Members |  |  |  |
| :---: | :---: | :---: | :---: |
| Depth | Round D | Square W\&H | Rectangular W\&L |
| 11" | 71/4" | 53/4" $\times 53 / 4{ }^{\prime \prime}$ | 3" $\times 13$ " |
| 14 " | 81/2" | $61 / 2^{\prime \prime} \times 61 / 2^{\prime \prime}$ | $3 " \times 14^{\prime \prime}, 6 " \times 8{ }^{\prime \prime}$ |
| 16" | 91/2" | $71 / 2{ }^{\prime \prime} \times 71 / 2^{\prime \prime}$ | $3 " \times 15$ " |
| 11-7/8' - up to 22' $14^{\prime \prime}$ - up to $26^{\prime} \quad 16^{\prime \prime}$ - up to 30' |  |  |  |



ADDITIONAL SERVICES
Full Stocking Inventory for Prompt Delivery
Job-site Delivery Available*
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Prompt Turnaround for Drawings
Commercial Joists - Architectural Glulams Available
Stamped Calcs Available*
iStruct ${ }^{\bullet}$ Software Available*
In-House/On-site Training*
*See Coastal Rep for more details


Power Column ${ }^{\circledR}$ - 1.9 MOE / 2400Fb

| 3-1/2" Wide |  |  | 5-1/2" Wide |  | 7"Wide |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\square$ |  |  |  |  |  |
| 3-1/2" | 5-1/2" | $7{ }^{\prime \prime}$ | 5-1/2" | $7{ }^{\prime \prime}$ | $7{ }^{\prime \prime}$ |
|  |  |  | ' Leng |  |  |

Power Preserved Column ${ }^{\circledR}$ - 1.9 MOE / 2100Fb


# THE COASTAL ADVANTAGE 

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| EMAIL | ewp@coastalfp.com |
| ADDRESS | 660 River Road • Bow, NH 03304 |

0 in


[^0]:    Uplift connections may be required.

